ERECTION NOTES

1. All bracing shown and provided by the Metal Building Provider for this building is required and shall be installed by the erector as a permanent part of the structure ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.10).
2. Temporary supports, such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined and furnished by the erector ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.10.3).
3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, grinding, welding or cutting, and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to the Metal Building Provider by the customer to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.14).
4. Erection tolerances are set forth in the "Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.13 note that individual members are considered plumb, level and aligned if the deviation does not exceed 1:500. Variations in finished overall dimensions of structural steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances.
   4.1. When crane support systems are part of the metal building system erection tolerances Section 6.8, Erection Tolerances, 2018 MBMA Metal Building Systems Manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The customer shall provide grout if required. The contractor erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met:
   5.1. Welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code - Steel or AWS D1.3 Structural Welding Code - Sheet Steel as applicable, for the processes, positions, and materials involved.
   5.2. All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not prequalified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.
6. All documentation and records shall be the responsibility of the customer.
7. Any claims or shortages by buyer must be made to the Metal Building Provider within seven (7) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed. All claims should be directed to the Metal Building Provider's Customer Service Department.
8. Claims for correction of alleged misfits will be disallowed unless the Metal Building Provider shall have received prior notice thereof and allowed reasonable inspection of such misfits. Ordinary inaccuracies of shop work shall not be construed as misfits. No part of the building may be returned or charges assessed for alleged misfits without prior approval from the Metal Building Provider.
9. Neither the Metal Building Provider nor the customer will cut, drill or otherwise alter their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the customer is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges" in the ANSI/AISC 303-16; Section 7.15).
10. The Metal Building Provider Field Modifications Policy:
    10.1. The Metal Building Provider will only be responsible for the field-modified parts designed and approved by the Metal Building Provider's Customer Service Department.
    10.2. Any field modifications designed by third parties may not be approved by the Metal Building Provider and may limit the Metal Building Provider's warranty and liability.
    10.3. The Metal Building Provider makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field-modified parts performed by third parties.
11. WARNING - SOME PANELS AND TRIM PARTS ARE FURNISHED WITH A PROTECTIVE PEEL-OFF FILM. PARTS PROVIDED WITH THIS FILM CANNOT BE EXPOSED TO SUNLIGHT WITHOUT FIRST REMOVING THE FILM. THIS FILM MUST BE REMOVED PRIOR TO INSTALLATION. FILM MUST ALSO BE REMOVED FROM ALL NON EXPOSED PARTS WITHIN SIX MONTHS FROM FILM APPLICATION OR IRREPARABLE DAMAGE WILL OCCUR TO THE SURFACE. CLAIMS WILL NOT BE ACCEPTED FOR THIS ISSUE.

RESPONSIBILITIES

1. The Metal Building Provider Customer, hereafter referred to as the "customer", obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The customer provides at his expense all plans and specifications required to obtain a building permit. It is the customer's responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
2. The customer is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the metal building system.
3. It is the responsibility of the customer to interpret all aspects of the End User's specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to the Metal Building Provider.
4. It is the responsibility of the Metal Building Provider to furnish the metal building system to meet the specifications including the design criteria and design loads incorporated by the Contractor into the Order Documents. The Metal Building Provider is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Documents.
5. The Metal Building Provider's standard specifications apply unless stipulated otherwise in the Contract Documents. The Metal Building Provider design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary notwithstanding. It is understood by both parties that the customer is responsible for clarifications of inclusions or exclusions from the Architectural plans.
6. In case of discrepancies between the Metal Building Provider's structural steel plans and plans for other trades, the Metal Building Provider's shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16; Section 3.3)
7. The customer is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by the Metal Building Provider and the Metal Building Provider's steel system are to be considered and coordinated by the customer. Specific design criteria concerning this interface between materials must be furnished by the customer before release for fabrication or the Metal Building Provider's assumptions will govern.
8. Foundations, anchor rods, and anchor rod embedment are designed, furnished, and set by the customer in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5.1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 303-16.
9. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the customer in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance requirements.
10. The Metal Building Provider does not investigate the influence of the metal building system on existing buildings or structures. The End Customer assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building system.

GENERAL SPECIFICATIONS

1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited.
2. Oil-canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefore is not a cause for rejection.
3. The Metal Building Provider's red-oxide and gray oxide primer are designed for short term field protection from exposure to ordinary atmospheric conditions.
4. All bolts are 1/2" x 1-1/4" A307 unless noted. Refer to the erection drawings for specific framing connections and the cross-section(s) for main frame connections.
5. Unless noted otherwise on the frame cross section(s), all bolted joints with ASTM F3125 Grade A325 bolts are specified as snug-tightened joints in accordance with the Specification for Structural Joints Using High-Strength Bolts, June 11, 2020. Installation inspection requirements for Snug-Tight Bolts (Specification for Structural joints, Section 9.1) is suggested.
6. Unless noted otherwise, all bolted connections are designed as bearing type connections with bolt threads not excluded from the shear plane.
7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, and insulation types other than 3" standard duty fiberglass blanket insulation, etc.

1. WW to provide design only for a transverse monorail crane that will be located at Frame Line 3. Crane specs below:
   a.Qty.1
   b.Crane Type:Hoist
   c.Rated Capacity:4000
   d.Service Class:C
   e.Operation Control:Electric
   f.Hoist & Trolly Weight:313 lbs
   g.Max. Wheel Load w/o impact 4400
   h.Length of Runway:40'
   i.Number of wheels per End Truck:4
   j.Runway length starts 30'-0" from each sidewall
   k.Runway beam by others to weigh no more than 40 lbs/ft

BUILDING DESIGN CODES

Building Code:	IBC 15
Hot-rolled version:	AISC 360-10
Cold-formed version:	AISI S100-12

GENERAL LOADS

Dead Load:	2.40 psf	
Roof Collateral Load:	1.00 psf	(Misc.)
Sprinkler Load:	0.00 psf	
Roof Live Load:	20.00 psf	
Tributary Live Load Reduction:	No	
Rainfall Intensity:	4.00 in/hr	(5-minute duration 5-year recurrence)

WIND LOAD

Wind Speed (3-sec gust) Vult:	120 mph	
Vasd:	93 mph	
V service:	76 mph	
Exposure Factor:	D	
Wind Condition:	Enclosed	
Internal Pressure Coefficient :	+/- 0.18	
Edge Zone Width:	10.00 Ft	

SNOW LOAD

Ground Snow Load (Pg):	35.00 psf	(No data snow load provided by Metal Building Outlet)
Roof Snow Load (Pg):	35.00 psf	
Importance Factor:	1.20	
Exposure Factor:	1.00	
Thermal Factor:	1.00	
Slope Factor:	1.00	

DEFLECTION CRITERIA

Main Frames Horizontal:	H/60	Roof Panels:	L/60
Main Frames Vertical:	L/180	Purlins:	L/180
Bearing Frame Rafter:	L/180	Wall Panels:	L/60
Endwall Columns:	L/120	Girts:	L/90
Main Frames Vertical:	L/400 (Frame Line-3)		

For components,claddings and MWFRS, deflections involving wind are based on 10 year serviceability wind pressures.

SEISMIC LOAD

Risk Category:	IV - Post	
Seismic Importance Factor (Ie):	1.5000	
Structural Response Acceleration (Ss):	0.1550	
Structural Response Acceleration(S1):	0.0530	
Site Class:	D	
Design Spectral Response (Sds):	0.1650	
Design Spectral Response (Sd1):	0.0850	
Seismic Design Category:	C	

	Lateral	Longitudinal
Framing Direction:		
Structural Syst:	'Structural Steel Systems Not Specifically Detailed for Seismic Resistance'	
Response Modification Factor(s) (R):	3.0	3.0
Deflection Amplification Factor(s):	3.0	3.0
Sesimic Response Coefficient(s) (Cs):	0.0827	0.0827
Design Base Shear V :	15.50	13.090
Analysis Procedure:	Equivalent Lateral Force	

Other Loads:
1. Loads due to 65'X17' Bifold Door applied at LEW between lines B-G (Door furnished by Others)
Bifold Door Weight 9928 Lbs
2. 4000 lbs Point load at the center of frame line 3 to support a stationary
3. Frame Line 3 has been designed for a maximum crane load of 6600 lb at support brackets. Support brackets are to be located 30'-0" from each sidewall and are not provided by MBP.
4. Rigid Frame 2 is designed to carry Big Ass Fan loads (2) 415 lbs

ROOF PANEL

Profile: Super Span X Gauge: 26 Color: SMP COOL WHITE
UL580 Class 90: Yes
Clip Type if Standing Seam: N/A

WALL PANEL

Profile: Super Span X Gauge: 26 Color: SMP COOL WHITE

WAINSCOT PANEL

Profile: Super Span X Gauge: 26 Color: SMP PATRIOT RED

BI-FOLD DOOR PANEL

Profile: Super Span X Gauge: 26 Color: SMP COOL WHITE

PRIMARY FRAMING

Built-Up & Hot-Rolled: Gray Oxide Primer

SECONDARY FRAMING

Purlins, Eave Struts:	Pre-Galvanized
Girts, Light Gage Columns:	Pre-Galvanized
Light Gage Jambs & Headers:	Pre-Galvanized
Base Channel Finish:	Pre-Galvanized

Hot-Dip Galvanizing conforms to the ASTM A123 specification.
Pre-Galvanized members conform to the ASTM A653, Grade 50, Coating G-90 specification.

APPROVAL SPECIFICATIONS

1. Approval of the Metal Building Provider drawings and/or calculations indicate that the Metal Building Provider has correctly interpreted the contract requirements. This approval constitutes the customer acceptance of the Metal Building Provider design, concepts, assumptions, and loadings.
2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which the Metal Building Provider will not be responsible.
3. Any changes made after the Metal Building Provider's customer has signed and returned the Metal Building Provider drawings and/or calculations and the project is released for fabrication shall be billed to the Metal Building Provider customer including material, engineering, and other costs. An additional fee may be charged if the project must be moved in the fabrication and/or the shipping schedule.
4. It is the responsibility of the customer to field verify all existing conditions prior to fabrication.
5. It is imperative that any changes to these drawings:
   5.1. Be made in contrasting ink.
   5.2. Be legible and unambiguous.
   5.3. Have all instances of changes clearly indicated.
6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the Metal Building Provider customer.
7. The Metal Building Provider reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.
8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between the Metal Building Provider and its customer are not binding on the Metal Building Provider unless subsequently specifically acknowledged and agreed to in writing by change order or separate documentation.
9. Waiving the approval process by designating the order "For Production" supercedes notes 1, 2, 5, 6, and 8 in this section, and constitutes the customer acceptance of the Metal Building Provider's design, concepts, assumptions, and loadings.

TRIM COLOR

Northen Gutter:	SMP ASH GRAY	Gauge:	26
Northen Rake:	SMP ASH GRAY	Gauge:	26
Corner:	SMP ASH GRAY	Gauge:	26
Accessory:	SMP ASH GRAY	Gauge:	26
Downspouts:	SMP ASH GRAY	Gauge:	26
Base Trim:	SMP ASH GRAY	Gauge:	26
Waiscot Trim:	SMP ASH GRAY	Gauge:	26
Bi-Fold Door Trim:	SMP COOL WHITE	Gauge:	26

Additional Notes:
The rigid frame at line 1 & 5 is designed as a non-expandable rigid frame. Corresponding frame reactions are calculated based upon actual tributary area.
-Mezzanine is completely independent of the metal building. building manufacturer design didn't consider any loads transfer/ support of the mezzanine or any of its components.
-The Monorail runway beam, brackets and brace angles are not provided by Metal Building Outlet

DRAWING INDEX

DATE	ISSUE	DESCRIPTION
11.23.22	A2	C1-Cover Sheet
11.23.22	A2	F1-Anchor Bolt Plan & Details
11.23.22	A2	F2-Anchor Bolt Reactions
11.23.22	A2	F3-Anchor Bolt Reactions
11.23.22	A2	P1 - Rigid Frame Elevation
11.23.22	A2	P2 - Rigid Frame Elevation
11.23.22	A2	P3 - Rigid Frame Elevation
11.23.22	A2	P4 - Rigid Frame Elevation
11.23.22	A2	P5 - Rigid Frame Elevation
11.23.22	A2	E1 - Roof Framing Plan
11.23.22	A2	E2 - Roof Sheeting Plan
11.23.22	A2	E3 - Endwall Elevation
11.23.22	A2	E4 - Endwall Elevation
11.23.22	A2	E5 - Sidewall Elevation
11.23.22	A2	E6 - Sidewall Elevation
11.23.22	A2	E7 - Section Details
11.23.22	A2	D1 - Standard Detail Page
11.23.22	A2	D2 - Standard Detail Page
11.23.22	A2	D3 - Standard Detail Page
11.23.22	A2	D4 - Standard Detail Page

Drawings listed under Drawings Schedule are submitted for approval. These drawings represent Metal Building Provider (MBP) interpretation of our scope of work. You are reviewing these drawings to confirm that MBP has correctly interpreted the project requirements. All dimensions, sections, details and notes require your review. All "clouded areas" must be reviewed and addressed/answered before your project is placed within the schedule. Please complete all field verifications prior to returning your approval drawings. The MBP is not responsible for checking the material and/or design compatibility of components not supplied by MBP. MBP may not match architectural/structural drawings and ***specifications.***

Please sign and return marked either:
"Approved as Submitted" (no changes or corrections and project is released for fabrication)

"Approved as Noted" (with corrections and/or changes clearly noted and project is released for fabrication)

"Revise and Resubmit" (with corrections and/or changes clearly noted and revised drawings will be resubmitted for approval)

☐ Approved As Submitted
☐ Approved As Noted
☐ Revised & Resubmit

Approval Signature: Date:
Print Name:
Desired Delivery Date:

The Engineer whose seal and signature appear on these documents represent Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Metal Building Provider, and excludes parts such as doors, windows, foundation design, and erection of the building.

Mustapha Chreeide
I am the author of this document
2022.12.01 10:33:13-06'00'

COLORADO LICENSED MUSTAPHA I. CHREEIDE 49784 PROFESSIONAL ENGINEER

12/1/2022

DRAWING STATUS

☒ FOR APPROVAL:
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☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.

METALBUILDING OUTLET CORP.
7651 SHAFFER PARKWAY LITTLETON, CO 80127

APPROVAL/REVIEWING AUTHORITY: PLEASE REVIEW APPROVAL DRAWINGS CAREFULLY

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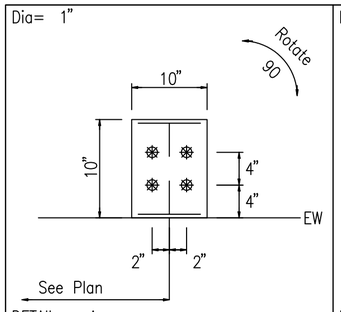
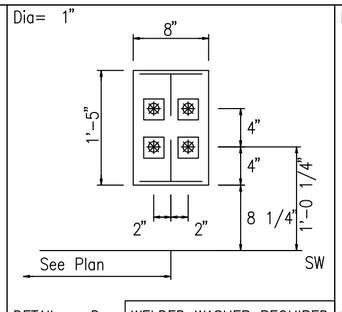
ISSUE	DATE	DESCRIPTION	BY	CHK
P1	07.01.22	FOR CONSTRUCTION PERMIT	KXJ	PNR
P2	07.19.22	REV. FOR CONSTRUCTION PERMIT	BCB	PNR
A1	09.08.22	FOR APPROVAL	BCB	PNR
A2	11.23.22	REV.FOR APPROVAL	SXS	PNR

SHEET DESCRIPTION: COVER SHEET
BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"
CUSTOMER: CLASSIC AVIATION
CUSTOMER LOCATION: CORTEZ, CO 81321
PROJECT REFERENCE: CLASSIC AVIATION
JOBSITE LOCATION: CORTEZ, CO 81321
JOBSITE COUNTY: MONTEZUMA

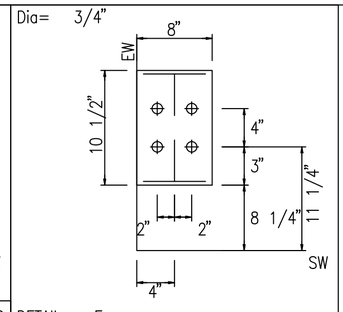
DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
SXS	PNR	11.23.22	AJF	9480-28780	C1	A2



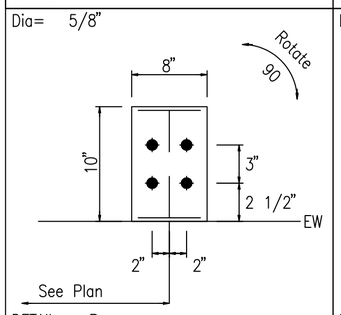
DETAIL A



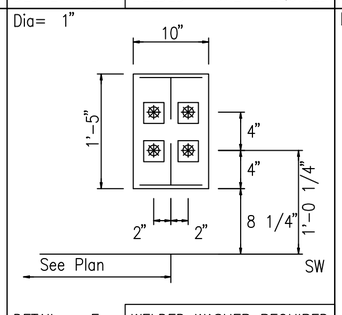
DETAIL D | WELDED WASHER REQUIRED



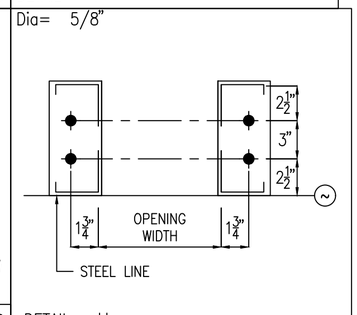
DETAIL F



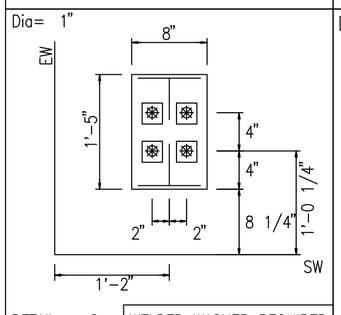
DETAIL B



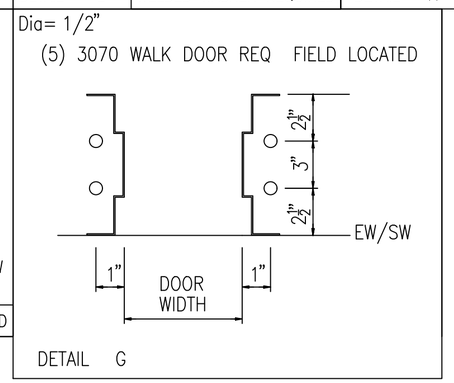
DETAIL E | WELDED WASHER REQUIRED



DETAIL H

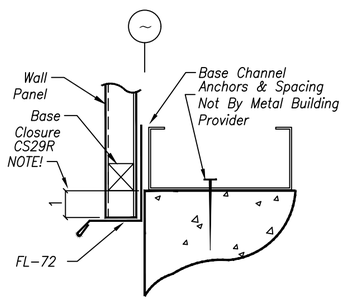


DETAIL C | WELDED WASHER REQUIRED



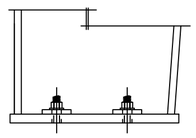
DETAIL G

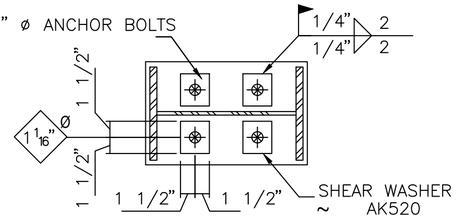
Horizontal leg of recess must remain flat or slope away from the building. Notch area indicates a recess for metal wall panels. Panels must not touch the bottom of the recess, which will void the warranty.



DETAIL @ SHEETING RECESS | NTS

NOTE:
USE AT DETAIL "C,D&E"

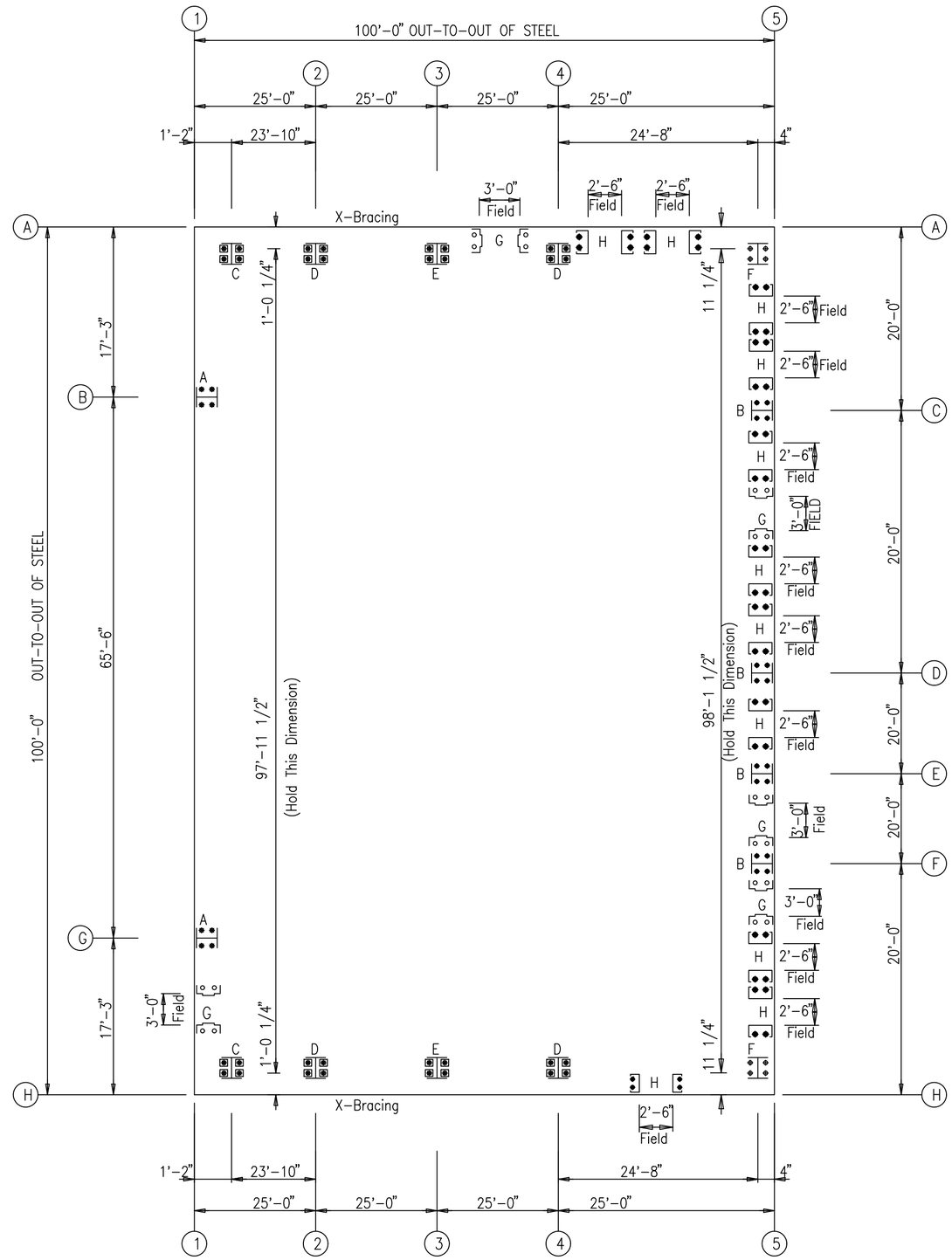




1. SHEAR WASHER ARE FURNISHED BY METAL BUILDING OUTLET.
2. SHEAR WASHER TO BE FIELD WELDED

SHEAR WASHER (FIELD WELDED)

Welded Washer Required
Only for Frame lines 1 to 4



NOTE: All Base Plates @ FINISHED FLOOR (U.N.)

ANCHOR BOLT PLAN

The Engineer whose seal and signature appear on these documents represent Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Metal Building Provider, and excludes parts such as doors, windows, foundation design, and erection of the building.

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METAL BUILDING OUTLET CORP.
7651 SHAFFER PARKWAY LITTLETON, CO 80127

ISSUE	DATE	DESCRIPTION	BY	CHK
0	07.01.22	FOR ERECTOR INSTALLATION	KXJ	PNR
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A1	09.08.22	FOR APPROVAL	BCB	PNR
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SHEET DESCRIPTION:	ANCHOR BOLT PLAN & DETAILS
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CUSTOMER LOCATION:	CORTEZ, CO 81321
PROJECT REFERENCE:	CLASSIC AVIATION
JOBSITE LOCATION:	CORTEZ, CO 81321
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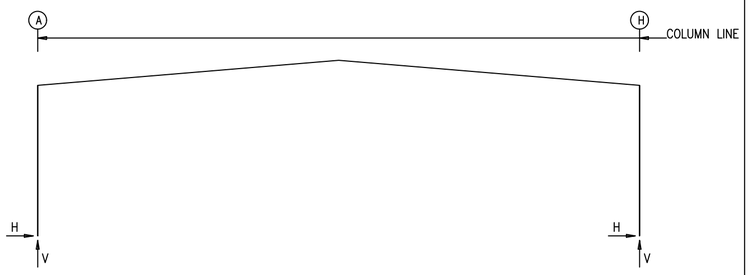
DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
SXS	PNR	11.23.22	AJF	9480-28780	F1	A2

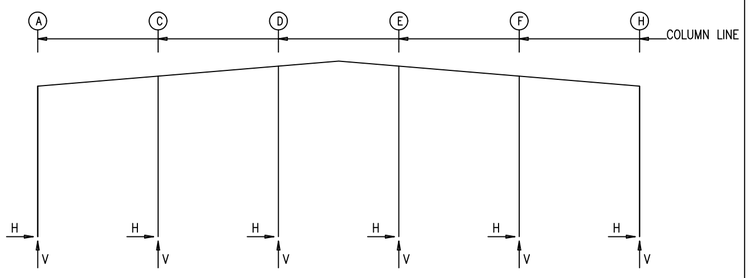
COLORADO LICENSED MUSTAPHA I. CAREEIDE 49784 PROFESSIONAL ENGINEER

12/1/2022

FRAME LINES: 1 2 3 4



FRAME LINES: 5



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Welded Washer Required Only for Frame lines 1 to 4

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
1	A	1	25.7	39.5	6	−4.3	−6.3	4	1.000	8.000	17.00	0.500	0.0
1	H	7	4.3	−6.3	1	−25.7	39.5	4	1.000	8.000	17.00	0.500	0.0
		1	−25.7	39.5	7	4.3	−6.3						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Welded Washer Required Only for Frame lines 1 to 4

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
2	A	1	32.2	52.0	6	−15.6	−21.2	4	1.000	8.000	17.00	0.750	0.0
					10	−6.8	−29.7						
2	H	7	15.6	−21.2	1	−32.2	52.0	4	1.000	8.000	17.00	0.750	0.0
		1	−32.2	52.0	12	6.8	−29.7						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Welded Washer Required Only for Frame lines 1 to 4

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
3	A	1	33.6	54.8	6	−12.6	−17.2	4	1.000	10.00	17.00	0.625	0.0
					10	−5.0	−26.0						
3	H	7	12.6	−17.2	1	−33.6	54.7	4	1.000	10.00	17.00	0.625	0.0
		1	−33.6	54.7	12	5.0	−26.0						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Welded Washer Required Only for Frame lines 1 to 4

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
4	A	1	32.1	51.6	6	−16.1	−21.7	4	1.000	8.000	17.00	0.625	0.0
4	H	7	16.1	−21.7	1	−32.1	51.6	4	1.000	8.000	17.00	0.625	0.0
		1	−32.1	51.6	7	16.1	−21.7						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
1	B	16	13.6	7.1	17	−5.4	7.1	4	1.000	10.00	10.00	0.500	0.0
		18	−1.2	12.1									
1	G	16	13.6	7.1	17	−5.4	7.1	4	1.000	10.00	10.00	0.500	0.0
		18	−1.2	12.1									
5	F *	19	5.1	−6.8	20	−4.6	−6.8						
		21	0.0	12.0	19	5.1	−6.8						
5	E *	22	5.4	−5.6	20	−4.9	−5.3						
		21	0.0	12.8	22	5.4	−5.6						
5	D *	23	5.4	−5.6	17	−4.9	−5.3						
		24	0.0	12.8	23	5.4	−5.6						
5	C *	25	5.1	−6.8	17	−4.6	−6.8						
		24	0.0	12.0	25	5.1	−6.8						

*See Rigid Frame Interior Column Reactions

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
1	A	11.2	15.9	0.4	0.7	8.0	13.1	14.1	22.9	−18.4	−26.5	−7.7	−17.8
1	H	−11.2	15.9	−0.4	0.7	−8.0	13.1	−14.1	22.9	7.7	−17.8	18.4	−26.5

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
1	A	−15.3	−18.2	−4.6	−9.5	−9.5	−25.0	−10.5	−19.3	−2.0	−0.9	2.0	0.9
1	H	4.6	−9.5	15.3	−18.2	10.5	−19.3	9.5	−25.0	−2.0	0.9	2.0	−0.9

Frame Line	Column Line	MIN_SNOW Horiz	MIN_SNOW Vert	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
1	A	9.7	15.7	11.1	21.7	11.1	12.5
1	H	−9.7	15.7	−11.1	12.5	−11.1	21.7

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2	A	3.7	7.0	0.8	1.3	15.9	25.0	27.8	43.8	−29.7	−42.3	−13.0	−29.2
2	H	−3.7	7.0	−0.8	1.3	−15.9	25.0	−27.8	43.7	13.0	−29.2	29.7	−42.3

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2	A	−23.6	−26.5	−6.9	−13.4	−14.9	−56.5	−16.4	−48.1	−1.4	−0.6	1.4	0.6
2	H	6.9	−13.4	23.6	−26.5	16.4	−48.1	14.9	−56.5	−1.4	0.6	1.4	−0.6

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	F2UNB_SL_L Horiz	F2UNB_SL_L Vert	F2UNB_SL_R Horiz	F2UNB_SL_R Vert
2	A	0.0	−6.0	19.0	30.0	21.8	41.5	21.8	23.8
2	H	0.0	−6.0	−19.0	30.0	−21.8	23.8	−21.8	41.5

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
3	A	3.9	7.7	2.7	3.3	15.5	25.0	27.1	43.8	−24.9	−36.4	−10.9	−25.9
3	H	−3.9	7.7	−2.7	3.2	−15.5	25.0	−27.1	43.8	10.9	−25.9	24.9	−36.4

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
3	A	−18.8	−20.6	−4.9	−10.1	−12.1	−51.0	−13.3	−44.4	−2.2	−1.0	2.2	1.0
3	H	4.9	−10.1	18.8	−20.6	13.3	−44.4	12.1	−51.0	−2.2	1.0	2.2	−1.0

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	F3CRANEA1 Horiz	F3CRANEA1 Vert	F3CRANEA2 Horiz	F3CRANEA2 Vert	F3UNB_SL_L Horiz	F3UNB_SL_L Vert	F3UNB_SL_R Horiz	F3UNB_SL_R Vert
3	A	0.0	−6.0	18.6	30.0	4.0	5.9	5.7	6.8	21.3	41.4	21.3	23.8
3	H	0.0	−6.0	−18.6	30.0	−5.0	5.5	−4.7	6.4	−21.3	23.8	−21.3	41.4

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
4	A	3.4	6.6	0.8	1.3	15.9	25.0	27.9	43.8	−30.2	−42.8	−13.3	−29.6
4	H	−3.4	6.6	−0.8	1.3	−15.9	25.0	−27.9	43.8	13.3	−29.6	30.2	−42.8

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
4	A	−24.1	−27.0	−7.2	−13.8	−15.3	−40.4	−16.8	−32.0	−1.4	−0.6	1.4	0.6
4	H	7.2	−13.8	24.1	−27.0	16.8	−32.0	15.3	−40.4	−1.4	0.6	1.4	−0.6

Frame Line	Column Line	MIN_SNOW Horiz	MIN_SNOW Vert	F4UNB_SL_L Horiz	F4UNB_SL_L Vert	F4UNB_SL_R Horiz	F4UNB_SL_R Vert
4	A	19.1	30.0	21.9	41.5	21.9	23.8
4	H	−19.1	30.0	−21.9	23.8	−21.9	41.5

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
5	A	0.0	0.9	0.0	0.1	0.2	2.5	0.4	4.4	−5.1	−9.3	5.2	0.5
5	H	0.0	0.9	0.0	0.1	−0.2	2.5	−0.4	4.4	−5.2	0.5	5.1	−9.3
5	C	0.0	1.2	0.0	0.3	0.0	5.1	0.0	8.9	0.0	−6.3	0.0	−11.0
5	D	0.0	1.2	0.0	0.3	0.0	5.1	0.0	8.9	0.0	−11.9	0.0	−4.9
5	E	0.0	1.2	0.0	0.3	0.0	5.1	0.0	8.9	0.0	−4.9	0.0	−11.9
5	F	0.0	1.2	0.0	0.3	0.0	5.1	0.0	8.9	0.0	−11.0	0.0	−6.3

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
5	A	−6.6	−7.1	3.8	2.8	3.1	−5.6	2.1	−5.3	−0.9	−1.4	0.9	1.4
5	H	−3.8	2.8	6.6	−7.1	−2.1	−5.3	−3.1	−5.6	−0.9	1.4	0.9	−1.4
5	C	0.0	−3.6	0.0	−8.2	0.0	−11.5	0.0	−4.2	0.0	1.7	0.0	−1.7
5	D	0.0	−8.8	0.0	−1.9	0.0	−9.9	0.0	−6.6	0.0	−0.5	0.0	0.5
5	E	0.0	−1.9	0.0	−8.8	0.0	−6.6	0.0	−9.9	0.0	0.5	0.0	−0.5
5	F	0.0	−8.2	0.0	−3.6	0.0	−4.2	0.0	−11.5	0.0	−1.7	0.0	1.7

Frame Line	Column Line	MIN_SNOW Horiz	MIN_SNOW Vert	F5UNB_SL_L Horiz	F5UNB_SL_L Vert	F5UNB_SL_R Horiz	F5UNB_SL_R Vert
5	A	0.2	3.0	0.2	4.0	0.2	1.5
5	H	−0.2	3.0	−0.2	1.5	−0.2	4.0
5	C	0.0	6.1	0.0	10.1	0.0	2.3
5	D	0.0	6.1	0.0	11.6	0.0	3.5
5	E	0.0	6.1	0.0	3.5	0.0	11.6
5	F	0.0	6.1	0.0	2.3	0.0	10.1

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Horz	Dead Vert	Wind Press Horz	Wind Suct Horz	Seis Long Horz
1	B	−1.1	11.9	−9.1	22.6	0.1
1	G	−1.1	11.9	−9.1	22.6	0.1

Frm Line	Col Line	Wind Press Horz	Wind Suct Horz	MIN_SNOW Horz	MIN_SNOW Vert	E2UNB_SL_L Horz	E2UNB_SL_L Vert	E2UNB_SL_R Horz	E2UNB_SL_R Vert
5	F	−7.7	8.5	0.0	6.7	0.0	10.7	0.0	2.8
5	E	−8.2	9.1	0.0	6.0	0.0	11.6	0.0	3.4
5	D	−8.2	9.1	0.0	6.0	0.0	3.4	0.0	11.6
5	C	−7.7	8.5	0.0	6.7	0.0	2.8	0.0	10.7

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Elev. (in)
1	B	16	13.6	7.1	17	−5.4	7.1	4	1.000	10.00	10.00	0.500	0.0
		18	−1.2	12.1									
1	G	16	13.6	7.1	17	−5.4	7.1	4	1.000	10.00	10.00	0.500	0.0
		18	−1.2	12.1									
5	F *	19	5.1	−6.8	20	−4.6	−6.8						
		21	0.0	12.0	19	5.1	−6.8						
5	E *	22	5.4	−5.6	20	−4.9	−5.3						
		21	0.0	12.8	22	5.4	−5.6						
5	D *	23	5.4	−5.6	17	−4.9	−5.3						
		24	0.0	12.8	23	5.4	−5.6						
5	C *	25	5.1	−6.8	17	−4.6	−6.8						
		24	0.0	12.0	25	5.1	−6.8						

*See Rigid Frame Interior Column Reactions

ANCHOR BOLT SUMMARY (GRADE 36)

Qty	Locate	Dia (in)	Type	Proj (in)
❄ 8	Endwall	1"	F1554	3.50
● 16	Endwall	5/8"	F1554	2.50
❄ 32	Frame	1"	F1554	3.50
⊕ 8	Frame	3/4"	F1554	3.00
● 44	Jamb	5/8"	F1554	2.50
○ 20	Walk Door	1/2"	F1554	2.00

BUILDING BRACING REACTIONS

Wall Loc	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	1							(h)
F_SW	H	2,3	18.0	16.6	6.5	6.0		
R_EW	5							(h)
B_SW	A	3,2	18.0	16.6	6.5	6.0		

(h)Rigid frame at endwall

DRAWING STATUS

☒ FOR APPROVAL:
These drawings, being for approval,are by definition not final and are for conceptual representation only.Their purpose is to confirm the proper interpretation of the project documents.Only drawings issued "For Erection installation" can be considered complete.

☐ FOR CONSTRUCTION PERMIT:
These drawings, being for permit,are by definition not final.
Only drawings issued "For Erection installation" can be considered complete.

☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.

METALBUILDING OUTLET CORP.
7651 SHAFFER PARKWAY LITTLETON, CO 80127

ISSUE	DATE	DESCRIPTION	BY	CHK
0	07.01.22	FOR ERECTOR INSTALLATION	KXJ	PNR
1	07.19.22	REV. FOR ERECTOR INSTALLATION	BCB	PNR
A1	09.08.22	FOR APPROVAL	BCB	PNR
A2	11.23.22	REV.FOR APPROVAL	SXS	PNR

The Engineer whose seal and signature appear on these documents represent Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Metal Building Provider, and excludes parts such as doors, windows, foundation design, and erection of the building.

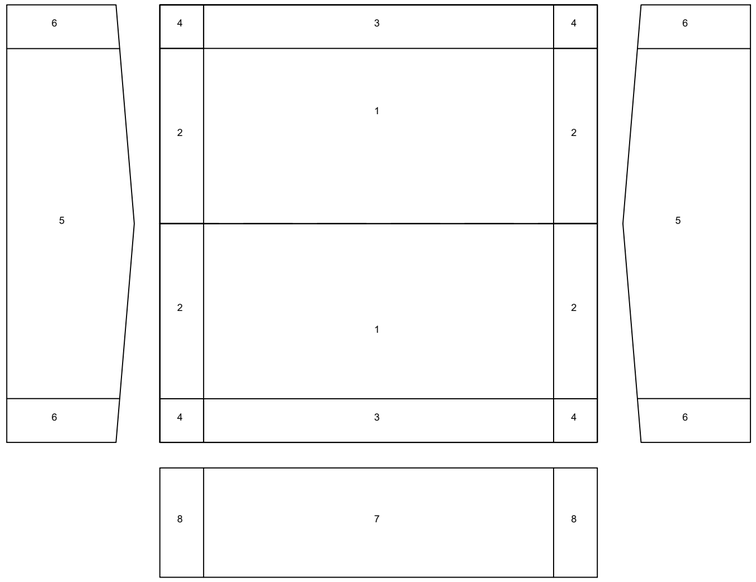
APPROVAL/REVIEWING AUTHORITY: PLEASE REVIEW APPROVAL DRAWINGS CAREFULLY

UNLESS NOTED OTHERWISE, IT WILL BE ASSUMED THAT ALL INFORMATION SHOWN ON THIS SET OF DRAWINGS HAVE THE AFFIRMATION OF THE APPROVAL/REVIEW AUTHORITY. FAILURE TO RESPOND TO CLOUDED AREAS AND AREAS TO VERIFY MAY RESULT IN ADDITIONAL COSTS AND/OR SCHEDULE DELAYS FOR WHICH METAL BUILDING PROVIDER WILL NOT BE RESPONSIBLE. FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION. ALL SUBSEQUENT CHANGES AFTER THE FIRST SUBMITTAL WILL BE CONSIDERED AS CONTRACTUAL CHANGES.

SHEET DESCRIPTION: ANCHOR BOLT REACTIONS					BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"
CUSTOMER: CLASSIC AVIATION					CUSTOMER LOCATION: CORTEZ, CO 81321
PROJECT REFERENCE: CLASSIC AVIATION					
JOBSITE LOCATION: CORTEZ, CO 81321					JOBSITE COUNTY: MONTEZUMA
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: F2 / ISSUE: A2

COLORADO LICENSED
MUSTAPHA I. CHREEDE
49784
PROFESSIONAL ENGINEER

12/1/2022



Components & Cladding

Zone	Width (ft)	Length (ft)	Pressure(psf) Member	Pressure(psf) Panel	Suction(psf) Member	Suction(psf) Panel
1			16.00	16.84	−37.90	−41.41
2		10.00	16.00	16.84	−44.72	−69.57
3	10.00		16.00	16.84	−44.72	−69.57
4	10.00	10.00	16.00	16.84	−44.72	−104.8
5			30.67	37.90	−33.83	−41.06
6	10.00		30.67	37.90	−36.20	−50.50
7			30.70	37.90	−33.80	−41.10
8	10.00		30.70	37.90	−36.17	−50.55

(+) wind towards surface
(−) wind away from surface

NOTES FOR REACTIONS

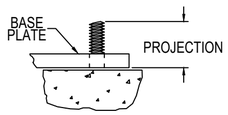
1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Loading conditions are:

1	Dead+Collateral+Snow+Slide_Snow
2	Dead+0.6Wind_Left1
3	Dead+0.6Wind_Right1
4	Dead+Collateral+0.75Snow+0.45Wind_Left2+0.75Slide_Snow
5	Dead+Collateral+0.75Snow+0.45Wind_Right2+0.75Slide_Snow
6	0.6Dead+0.6Wind_Left1
7	0.6Dead+0.6Wind_Right1
8	0.6Dead+0.6Wind_Left2
9	0.6Dead+0.6Wind_Right2
10	0.6Dead+0.6Wind_Long1L
11	0.6Dead+0.6Wind_Long1R
12	0.6Dead+0.6Wind_Long2L
13	0.6Dead+0.6Wind_Long2R
14	Dead+Collateral+F5UNB_SL_L
15	Dead+Collateral+F5UNB_SL_R
16	0.6Dead+0.6Wind_Right2+0.6Wind_Suction
17	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
18	1.02Dead+1.02Collateral+0.52Seismic_LongR+0.75E1UNB_SL_R
19	0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
20	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
21	Dead+Collateral+E2UNB_SL_L
22	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
23	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
24	Dead+Collateral+E2UNB_SL_R
25	0.6Dead+0.6Wind_Suction+0.6Wind_Long2L

GENERAL NOTES

1. All anchor bolts (by others) to have nuts and flat washers.
2. All anchor bolts are designed to full S.A.E. diameters with cut threads. No substitutions are allowed.
3. The Metal Building Provider is not responsible for the design, materials and workmanship of the foundation. Anchor bolt plans prepared by the Metal Building Provider are intended to show only location, diameter, and projection of anchor bolts required to attach the Metal Building System to the foundation. The Metal Building Provider is responsible for providing to the Builder the loads imposed by the Metal Building System on the foundation. It is the responsibility of the End Customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. This is typically the responsibility of the Design Professional or Engineer of Record, which is another reason that their involvement in the Construction Project from the outset is highly recommended. (2012 MBMA Metal Building Systems Manual, Section 3.2.2)
4. The projection is based from the bottom of the base plate. Adjustments must be made for grout and/or leveling plates.

THREADED ANCHOR BOLT



NOTE: PROJECTION BASED FROM BOTTOM OF BASE PLATE. ADJUSTMENTS SHOULD BE MADE FOR GROUT AND/OR LEVELING PLATES.

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☐ FOR ERECTOR INSTALLATION:
Final drawings for construction.

METALBUILDING OUTLET CORP.
7651 SHAFFER PARKWAY LITTLETON, CO 80127

ISSUE	DATE	DESCRIPTION	BY	CHK
0	07.01.22	FOR ERECTOR INSTALLATION	KXJ	PNR
1	07.19.22	REV. FOR ERECTOR INSTALLATION	BCB	PNR
A1	09.08.22	FOR APPROVAL	BCB	PNR
A2	11.23.22	REV.FOR APPROVAL	SXS	PNR

SHEET DESCRIPTION: ANCHOR BOLT REACTIONS					BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"	
CUSTOMER: CLASSIC AVIATION					CUSTOMER LOCATION: CORTEZ, CO 81321	
PROJECT REFERENCE: CLASSIC AVIATION						
JOBSITE LOCATION: CORTEZ, CO 81321					JOBSITE COUNTY: MONTEZUMA	
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: F3	ISSUE: A2

COLORADO LICENSED
MUSTAPHA I. CAMEEIDE
49784
PROFESSIONAL ENGINEER

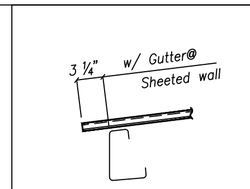
12/1/2022

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	1"	3 1/4"	8"	1"	3'-11 1/4"
SP-2	4	4	4	A325	3/4"	2"	8"	1/2"	4'-3"
SP-3	4	4	4	A325	3/4"	2 1/4"	8"	5/8"	4'-3"

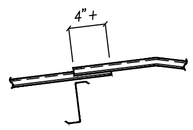
BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	8"	1/2"	1'-5"

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides

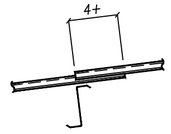
C - 56FBB
A - FB2214
D - 3X3X1/4"



DETAIL @ PANEL END



DETAIL @ RIDGE CAP

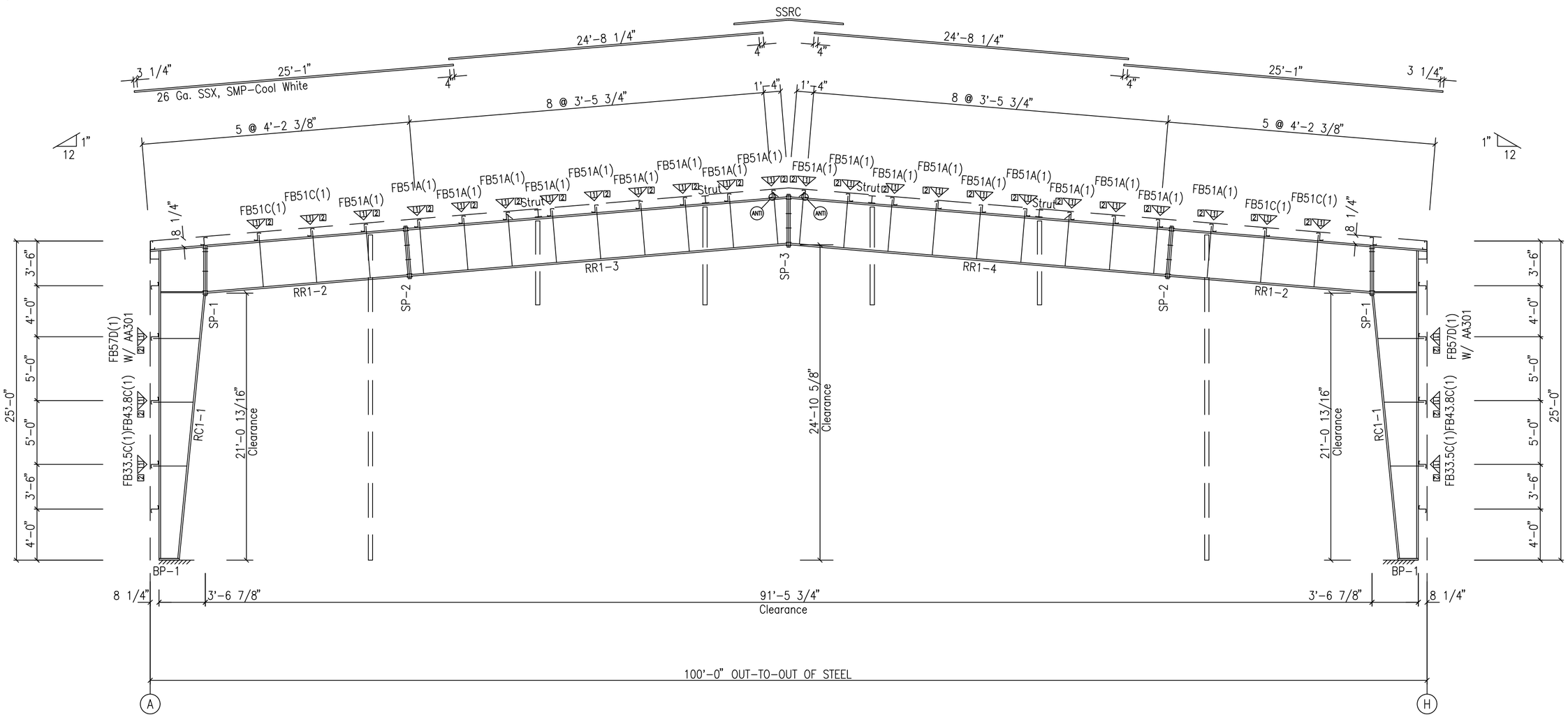


DETAIL @ PANEL SPLICE

MEMBER TABLE				
Mark	Web Depth Start/End	Web Plate Thick	Outside Flange W x Thk	Inside Flange W x Thk
RC1-1	16.0/41.1	0.250	8 x 3/8"	8 x 3/8"
	41.1/42.0	0.250	8 x 3/8"	8 x 1/2"
			8 x 5/8"	
RR1-2	42.0/42.0	0.313	8 x 3/8"	8 x 1/2"
				8 x 3/8"
RR1-3	42.0/42.0	0.250	8 x 1/4"	8 x 1/4"
	42.0/42.0	0.250	8 x 5/16"	8 x 1/4"
RR1-4	42.0/42.0	0.250	8 x 5/16"	8 x 1/4"
	42.0/42.0	0.250	8 x 1/4"	8 x 1/4"

CONNECTION PLATES	
☐ ID	Mark/Part
1	AK226
2	AK230

NOTE:
The rigid frame at lines 1 is designed as a non-expandable rigid frame. Corresponding frame reactions are calculated based upon actual tributary area.



RIGID FRAME ELEVATION: FRAME LINE 1

BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.

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METALBUILDING OUTLET CORP.
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A2	11.23.22	REV.FOR APPROVAL	SXS	PNR

SHEET DESCRIPTION: RIGID FRAME ELEVATION					BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"	
CUSTOMER: CLASSIC AVIATION					CUSTOMER LOCATION: CORTEZ, CO 81321	
PROJECT REFERENCE: CLASSIC AVIATION						
JOBSITE LOCATION: CORTEZ, CO 81321					JOBSITE COUNTY: MONTEZUMA	
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: P1	ISSUE: A2

COLORADO LICENSED
MUSTAPHA I. CHEEIDE
49784
PROFESSIONAL ENGINEER

12/1/2022

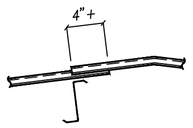
SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	1 1/4	4"	8"	1 1/4"	3'-11 1/2"
SP-2	4	4	2	A325	3/4"	2 1/4"	8"	5/8"	2'-9"
SP-3	4	4	2	A325	1"	2 1/2"	8"	5/8"	3'-9 1/4"

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	8"	3/4"	1'-5"

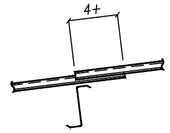
FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides

C - 56FBB
A - FB2214

DETAIL @ PANEL END



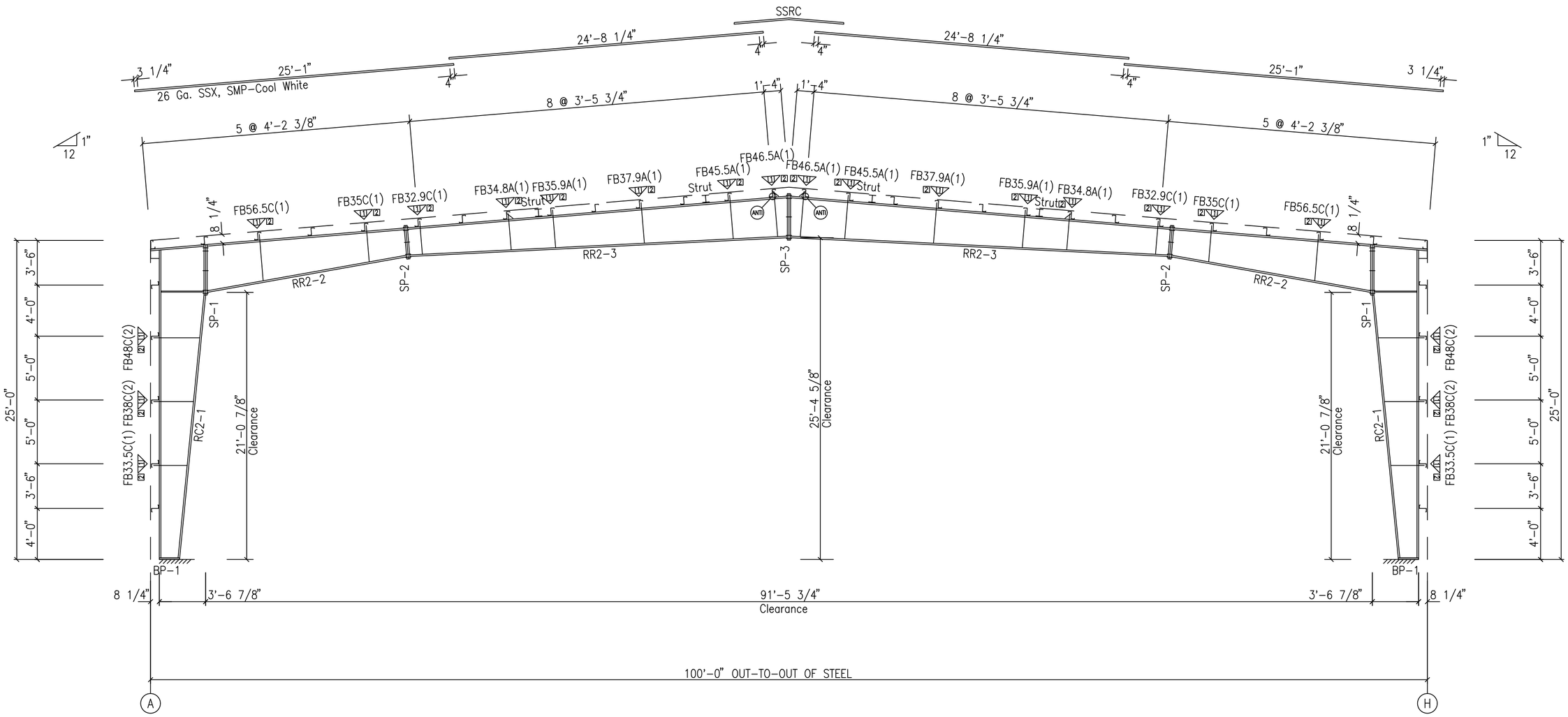
DETAIL @ RIDGE CAP



DETAIL @ PANEL SPLICE

MEMBER TABLE				
Mark	Web Depth Start/End	Web Plate Thick	Outside Flange W x Thk	Inside Flange W x Thk
RC2-1	16.0/28.6	0.250	8 x 1/4"	8 x 1/2"
	28.6/42.0	0.313	8 x 5/16"	8 x 5/8"
			10 x 5/8"	
RR2-2	42.0/24.1	0.313	8 x 3/8"	8 x 1/2"
			8 x 5/16"	8 x 3/8"
RR2-3	24.0/32.1	0.250	8 x 3/8"	8 x 5/16"
	32.1/36.0	0.188	8 x 1/2"	8 x 1/4"
				8 x 5/16"

CONNECTION PLATES	
☐ ID	Mark/Part
1	AK226
2	AK230



RIGID FRAME ELEVATION: FRAME LINE 2

BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.

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METALBUILDING OUTLET CORP.
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SHEET DESCRIPTION: RIGID FRAME ELEVATION					BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"	
CUSTOMER: CLASSIC AVIATION					CUSTOMER LOCATION: CORTEZ, CO 81321	
PROJECT REFERENCE: CLASSIC AVIATION						
JOBSITE LOCATION: CORTEZ, CO 81321					JOBSITE COUNTY: MONTEZUMA	
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: P2	ISSUE: A2

COLORADO LICENSED
MUSTAPHA I. CHREEIDE
49784
PROFESSIONAL ENGINEER

12/1/2022

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	4	A325	1 1/4	4"	10"	1 1/4"	4'-1 1/4"
SP-2	4	4	4	A325	3/4"	2"	10"	1/2"	4'-5"
SP-3	4	4	4	A325	1"	2 3/4"	10"	3/4"	4'-5 1/4"

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	10"	5/8"	1'-5"

▽FLANGE BRACES: FBxx (1 or 2)
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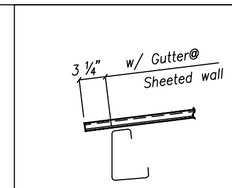
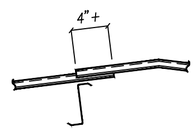
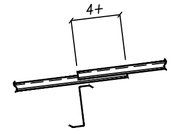
(1) One Side; (2) Two Sides
C – 56FBB
A – FB2214



DETAIL @ PANEL END



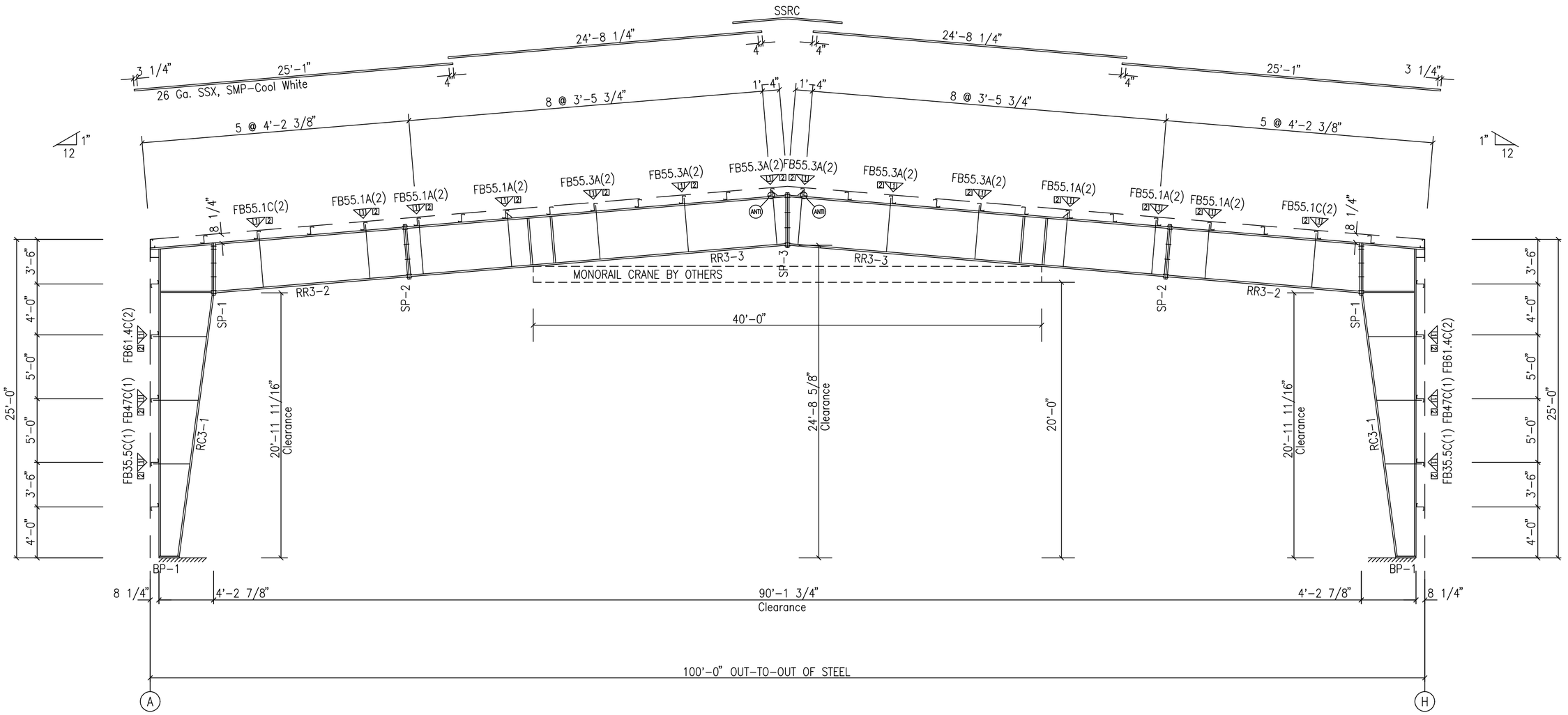
DETAIL @ RIDGE CAP



DETAIL @ PANEL SPLICE

MEMBER TABLE				
Mark	Web Depth Start/End	Web Plate Thick	Outside Flange W x Thk	Inside Flange W x Thk
RC3-1	16.0/32.5	0.250	10 x 3/8"	10 x 3/8"
	32.5/50.0	0.313	10 x 3/8"	10 x 1/2"
			10 x 1/2"	
RR3-2	44.0/44.0	0.313	10 x 3/8"	10 x 1/2"
				10 x 3/8"
RR3-3	44.0/44.0	0.313	10 x 3/8"	10 x 3/8"
	44.0/44.0	0.250	10 x 3/8"	10 x 3/8"

CONNECTION PLATES	
☐ ID	Mark/Part
1	AK226
2	AK230



RIGID FRAME ELEVATION: FRAME LINE 3

BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.

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SHEET DESCRIPTION: RIGID FRAME ELEVATION					BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"	
CUSTOMER: CLASSIC AVIATION					CUSTOMER LOCATION: CORTEZ, CO 81321	
PROJECT REFERENCE: CLASSIC AVIATION						
JOBSITE LOCATION: CORTEZ, CO 81321					JOBSITE COUNTY: MONTEZUMA	
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: P3	ISSUE: A2

COLORADO LICENSED MUSTAPHA I. CAMEEDE 49784 PROFESSIONAL ENGINEER

12/1/2022

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	1 1/4"	4"	8"	1 1/4"	3'-11 1/2"
SP-2	4	4	2	A325	3/4"	2 1/4"	8"	5/8"	2'-9"
SP-3	4	4	2	A325	1"	2 1/2"	8"	5/8"	3'-9 1/4"

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	8"	5/8"	1'-5"

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
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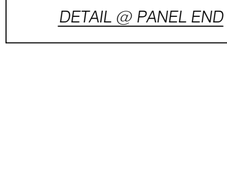
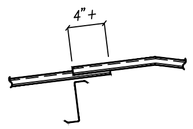
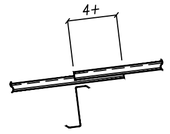
A - FB2214



DETAIL @ PANEL END



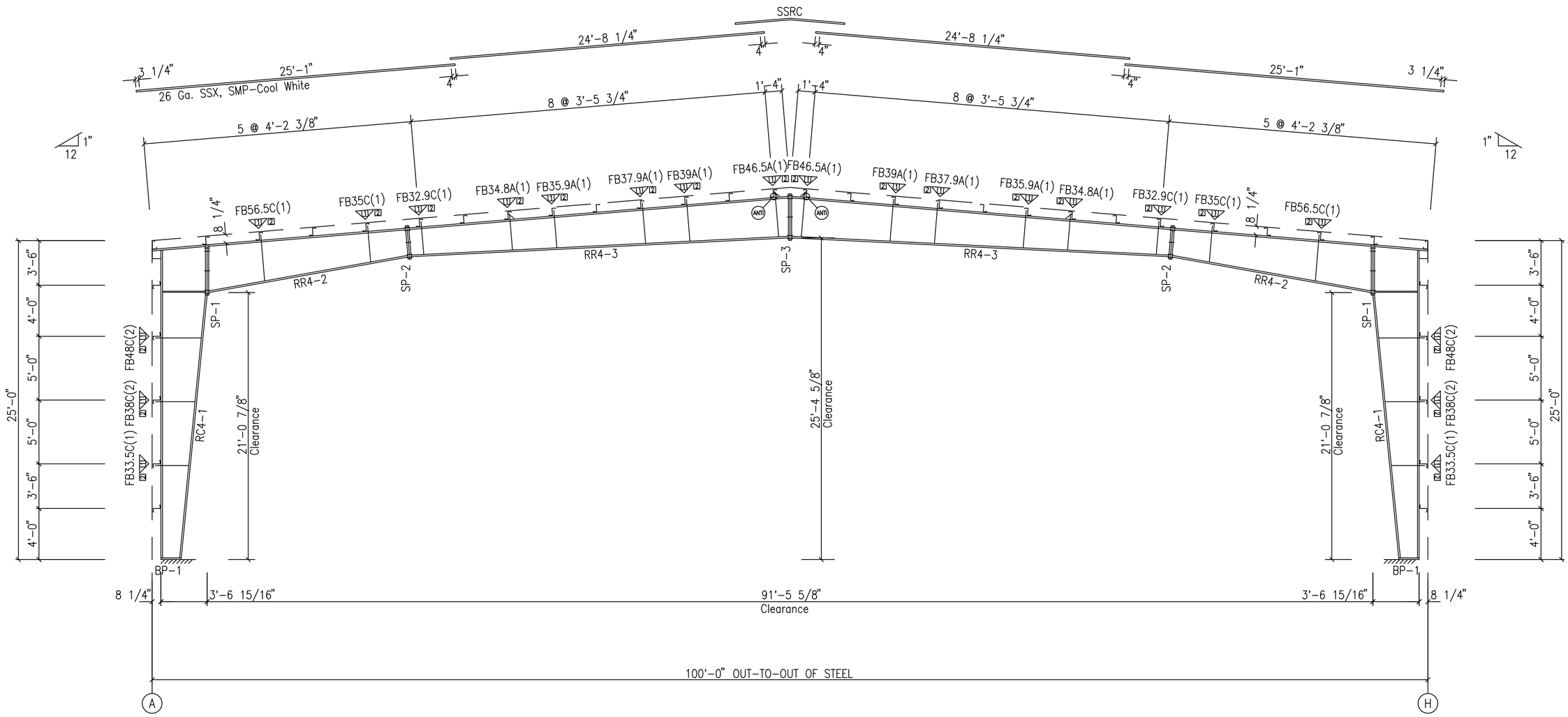
DETAIL @ RIDGE CAP



DETAIL @ PANEL SPLICE

MEMBER TABLE				
Mark	Web Depth Start/End	Web Plate Thick	Outside Flange W x Thk	Inside Flange W x Thk
RC4-1	16.0/28.6	0.250	8 x 5/16"	8 x 1/2"
	28.6/42.0	0.313	8 x 5/16" 10 x 5/8"	8 x 5/8"
RR4-2	42.0/24.1	0.313	8 x 3/8" 8 x 5/16"	8 x 1/2" 8 x 3/8"
RR4-3	24.0/32.1	0.250	8 x 3/8"	8 x 5/16"
	32.1/36.0	0.188	8 x 1/2"	8 x 1/4" 8 x 5/16"

CONNECTION PLATES	
☐ ID	Mark/Part
1	AK226
2	AK230



RIGID FRAME ELEVATION: FRAME LINE 4

BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

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A1	09.08.22	FOR APPROVAL	BCB	PNR
A2	11.23.22	REV.FOR APPROVAL	SXS	PNR

SHEET DESCRIPTION: RIGID FRAME ELEVATION					BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"	
CUSTOMER: CLASSIC AVIATION					CUSTOMER LOCATION: CORTEZ, CO 81321	
PROJECT REFERENCE: CLASSIC AVIATION						
JOBSITE LOCATION: CORTEZ, CO 81321					JOBSITE COUNTY: MONTEZUMA	
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: P4	ISSUE: A2

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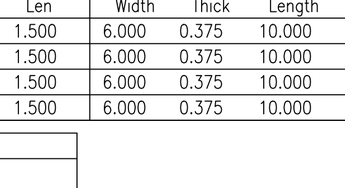
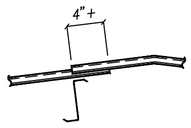
SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	3/4"	2"	6"	1/2"	1'-11"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-10 3/4"
SP-3	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-11"

CAP PLATES							
Col Id	Bolt Qnt	Type	Dia	Len	Plate Size Width	Thick	Length
EC-8	4	A325	0.625	1.500	6.000	0.375	10.000
EC-7	4	A325	0.625	1.500	6.000	0.375	10.000
EC-6	4	A325	0.625	1.500	6.000	0.375	10.000
EC-5	4	A325	0.625	1.500	6.000	0.375	10.000

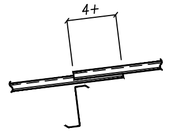
BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	8"	3/8"	10 1/2"
BP-2	8"	3/8"	10"



DETAIL @ PANEL END



DETAIL @ RIDGE CAP



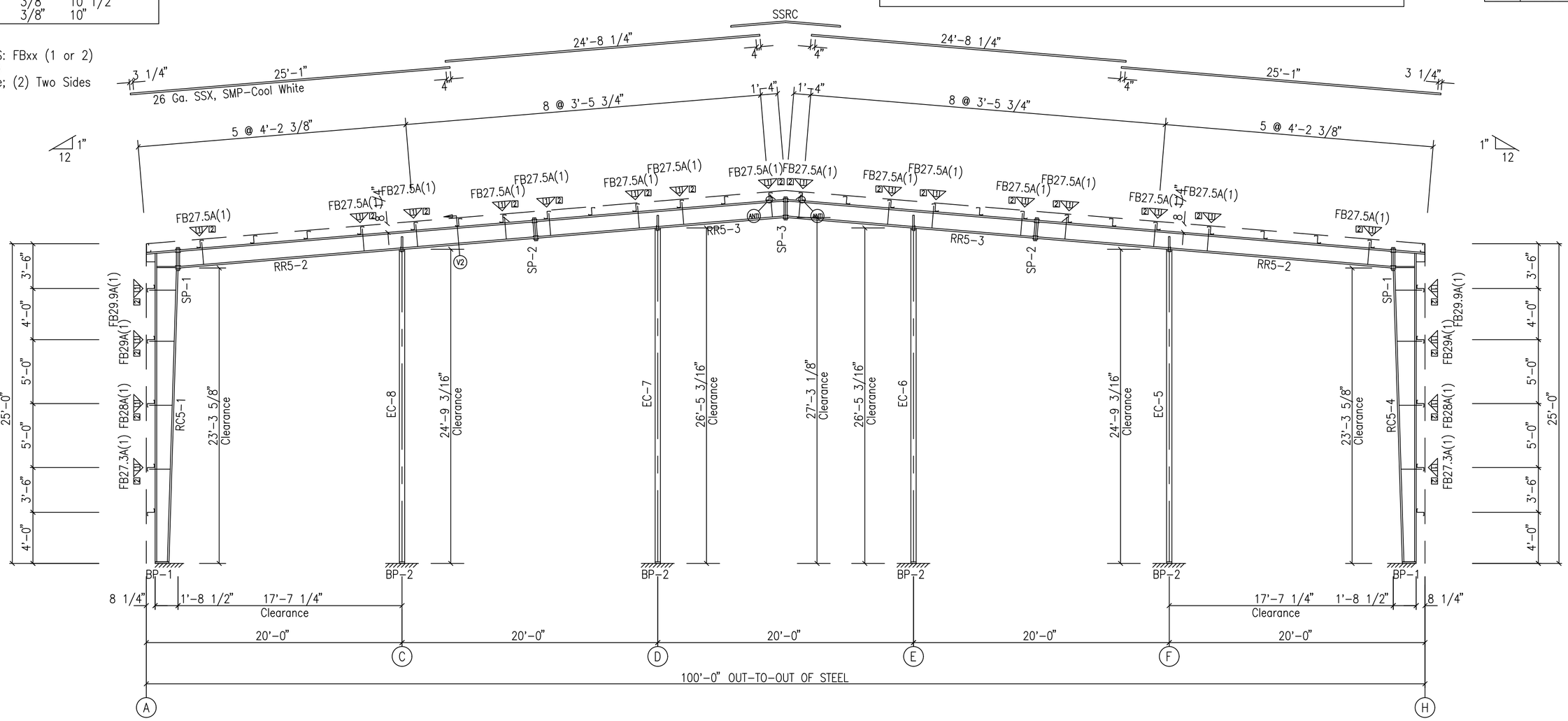
DETAIL @ PANEL SPLICE

MEMBER TABLE				
Mark	Web Depth Start/End	Web Plate Thick	Outside Flange W x Thk	Inside Flange W x Thk
RC5-1	10.0/14.4	0.135	5 x 1/4"	5 x 1/4"
	14.4/20.0	0.164	5 x 1/4"	5 x 1/4"
			5 x 1/4"	
RR5-2	14.0/14.0	0.135	5 x 1/4"	5 x 1/4"
	14.0/14.0	0.135	5 x 1/4"	5 x 1/4"
RR5-3	14.0/14.0	0.135	5 x 1/4"	5 x 1/4"
RC5-4	20.0/14.4	0.164	5 x 1/4"	5 x 1/4"
	14.4/10.0	0.135	5 x 1/4"	5 x 1/4"
			5 x 1/4"	
EC-8	W10541			
EC-7	W10541			
EC-6	W10541			
EC-5	W10541			

NOTE:
The rigid frame at lines 5 is designed as a non-expandable rigid frame. Corresponding frame reactions are calculated based upon actual tributary area.

CONNECTION PLATES	
☐ ID	Mark/Part
1	AK226
2	AK230

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A – FB2214



RIGID FRAME ELEVATION: FRAME LINE 5

BOLT TIGHTENING (Snug-Tight)

All bolted joints with ASTM F3125 Grade A325 bolts are specified as Snug-Tightened Joints in accordance with the Specification of Structural Joints Using High-Strength Bolts, June 11, 2020, installation as given in Section 7.1 Washers are not required for Snug-Tightened Joints using standard standard size holes per Section 6.1 of the Specification

Pretensioning methods, including Turn-of-Nut, calibrated wrench, twist-off tension control bolts or direct tension indicator are not required. Installation inspection requirements for Snug-Tight Bolt is found in Section 9.1 of the Specification.

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METALBUILDING OUTLET CORP.
7651 SHAFFER PARKWAY LITTLETON, CO 80127

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PROJECT REFERENCE: CLASSIC AVIATION
JOBSITE LOCATION: CORTEZ, CO 81321
JOBSITE COUNTY: MONTEZUMA

DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
SXS	PNR	11.23.22	AJF	9480-28780	P5	A2

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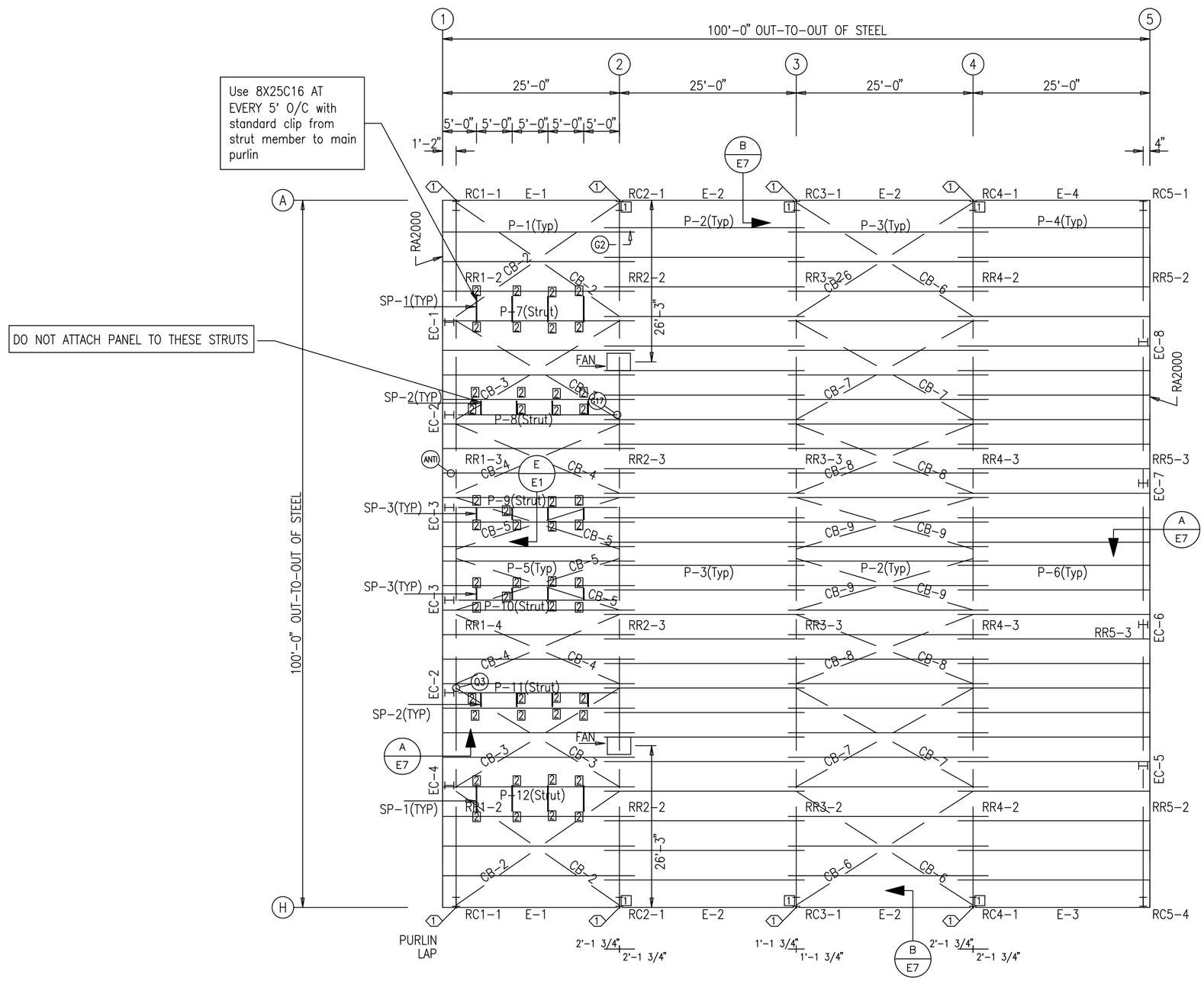
MUSTAPHA I. CHREIEDE
49784
PROFESSIONAL ENGINEER

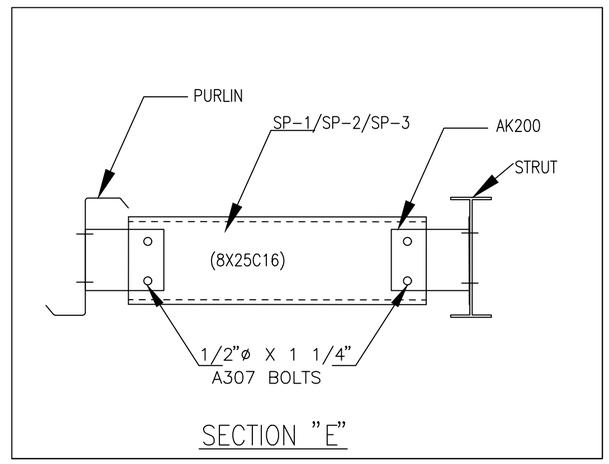
12/1/2022

SPECIAL BOLTS ROOF PLAN					
◇ ID	QUAN	TYPE	DIA	LENGTH	WASH
1	4	A307	1/2"	1 1/4"	0

MEMBER TABLE ROOF PLAN	
MARK	PART
P-1	8X25Z12
P-2	8X25Z12
P-3	8X25Z12
P-4	8X25Z12
P-5	8X25Z12
P-6	8X25Z12
P-7	W8X18
P-8	W8X18
P-9	W8X18
P-10	W8X18
P-11	W8X18
P-12	W8X18
E-1	8ES141
E-2	8ES141
E-3	8ES141
E-4	8ES141
CB-2	0.63_ROD
CB-3	0.50_ROD
CB-4	0.50_ROD
CB-5	0.50_ROD
CB-6	0.63_ROD
CB-7	0.50_ROD
CB-8	0.50_ROD
CB-9	0.50_ROD
SP-1	8X25C16
SP-2	8X25C16
SP-3	8X25C16

CONNECTION PLATES ROOF PLAN	
□ID	MARK/PART
1	AK106
2	AK200





SECTION "E"

ROOF FRAMING PLAN

UL580, CLASS 90 CONST. NUMBER 167

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7651 SHAFFER PARKWAY LITTLETON, CO 80127

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JOBSITE LOCATION: CORTEZ, CO 81321					JOBSITE COUNTY: MONTEZUMA	
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: E1	ISSUE: A2

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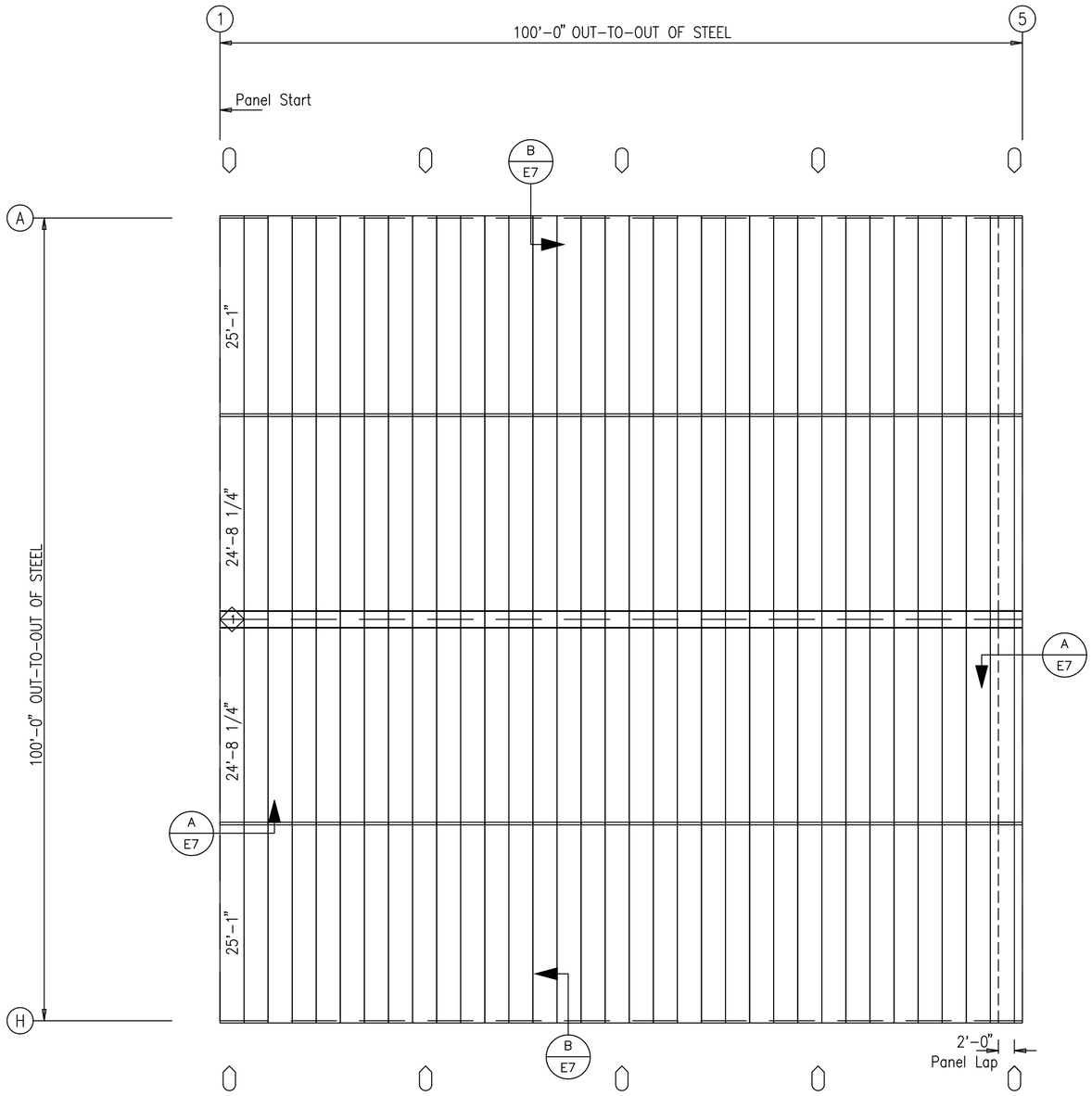
12/1/2022

DOWNSPOUT LOCATIONS

ROOF SHEETING TRIM TABLE			
◇ID	PART	LENGTH	QTY
1	SSRC30	3'-0"	34



ROOF SHEETING PLAN

PANELS: 26 Ga. SSX – Cool White

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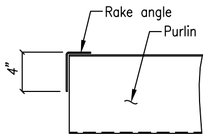
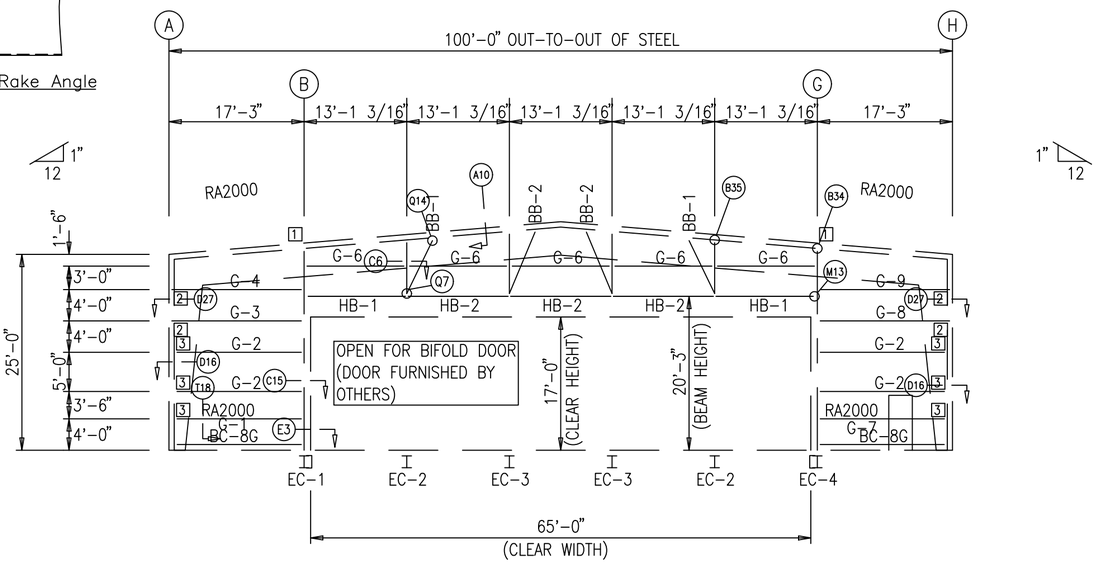
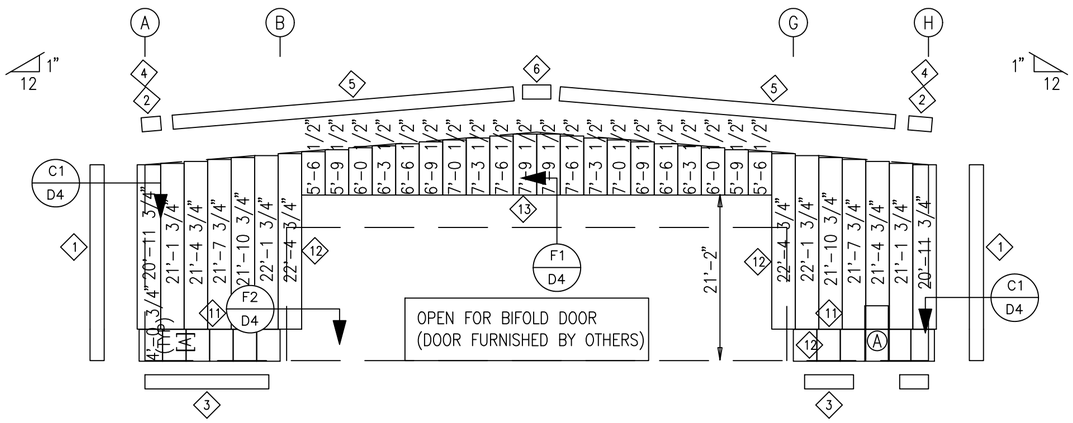


Detail at Rake Angle



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. SSX – SMP Cool White

[A] PANELS: 26 Ga. SSX –SMP– Patriot Red

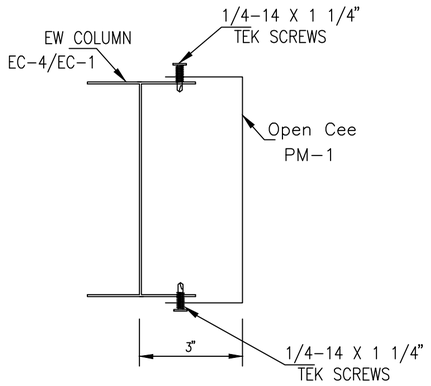
NOTE:
FEILD CUT IS REQUIRED FOR PANELS

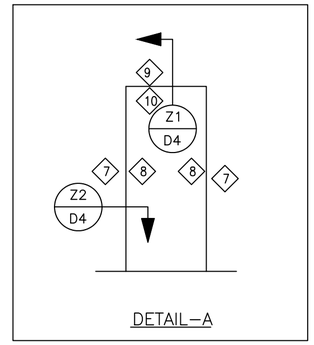
MEMBER TABLE				
Mark	Web Depth	Web Plate Thick	Outside Flange W x Thk	Inside Flange W x Thk
EC-1/EC-4	8.75	0.1644	6 x 5/8"	6 x 5/8"

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
EC-1/FRAME	8	A325	5/8"	1 1/2"
EC-2/FRAME	2	A325	3/4"	1 3/4"
EC-3/FRAME	2	A325	3/4"	1 3/4"
EC-4/FRAME	8	A325	3/4"	1 3/4"
Back Braces	1	A325	3/4"	1 3/4"

CONNECTION PLATES FRAME LINE 1	
☐ID	MARK/PART
1	AD414
2	AD249
3	AK244

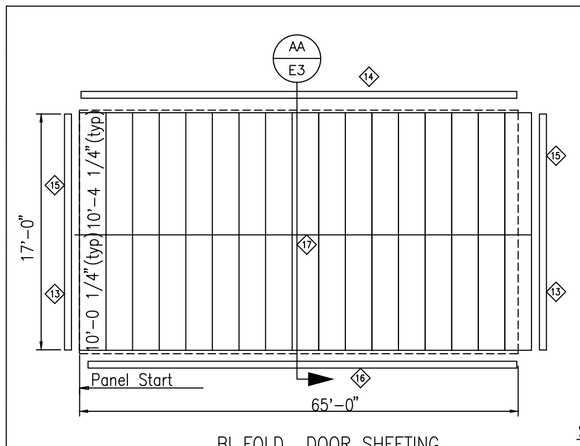
TRIM TABLE – THIS WALL ONLY FRAME LINE –1		
◇ID	PART	LENGTH
1	CT-102	13'-0"
2	SF-6	12'-0"
3	FL-72	10'-3"
4	SF-4	10'-3"
5	SF-2	20'-3"
6	SF-5	8"
7	MT-116B	7'-6"
8	JT-101	7'-6"
9	MT-116B	3'-8"
10	HT-101	3'-8"
11	WT-101	20'-3"
12	JT-101	12'-3"
13	JT-101	20'-3"





DETAIL–A

MEMBER TABLE FRAME LINE 1	
MARK	PART
HB-1	HSS6X6X1/2"
HB-2	HSS6X6X1/2"
BB-1	L4X4X1/4
BB-2	L4X4X1/4
EC-1	BUILTUP
EC-2	W10X12
EC-3	W10X12
EC-4	BUILTUP
G-1	8X25Z14
G-2	8X25Z12
G-3	8X25Z14
G-4	8X25Z12
G-6	8X25Z16
G-7	8X25Z16
G-8	8X25Z14
G-9	8X25Z12
PM-1	PM-1



TRIM TABLE HYDRAULIC DOOR		
◇ID	MARK	LENGTH
14	MT-113	20'-3"
15	MT-113	10-3"
13	MT-113	10-3"
16	FL-72	10-3"
17	SF-10	20-3"

BI FOLD DOOR SHEETING

PANELS: 26 Ga. SSX –SMP COOL WHITE

NOTE:
Field cut Panel As Required

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self-tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.
14. Sub-jambs for overhead doors, if required, is not furnished by Metal Building Provider

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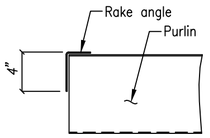
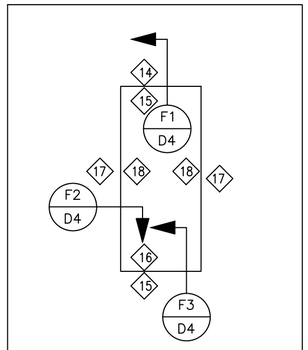
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: E3	ISSUE: A2

COLORADO LICENSED
MUSTAPHA I. CHEEDE
49784
PROFESSIONAL ENGINEER

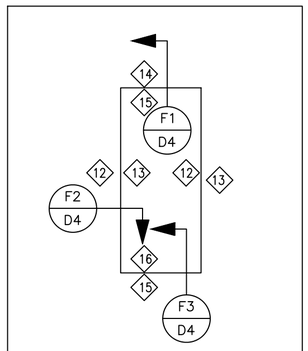
12/1/2022



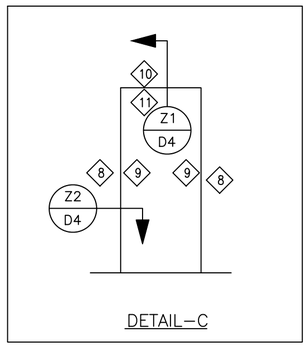
Detail at Rake Angle



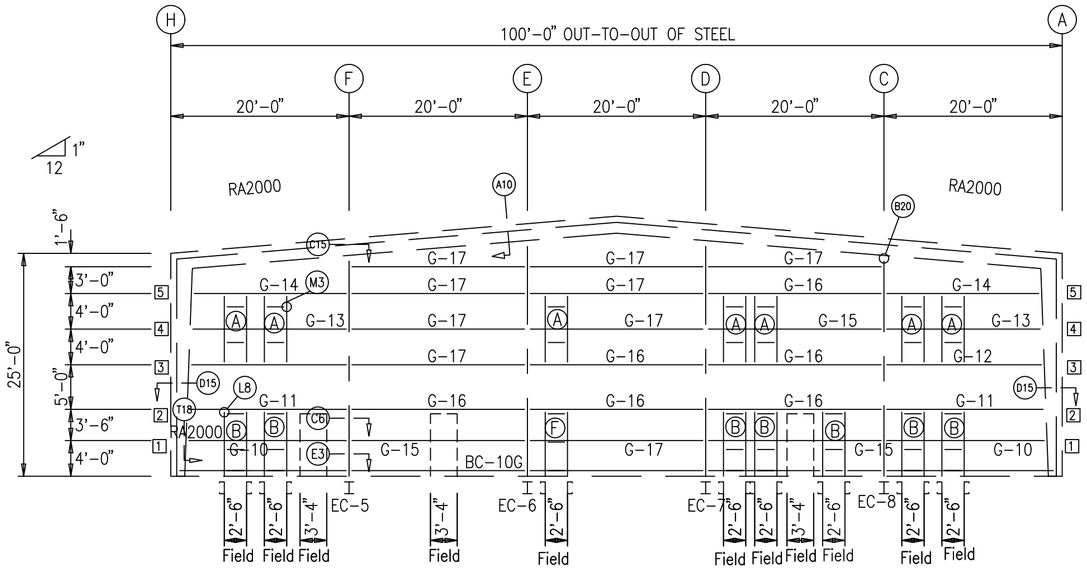
DETAIL–E



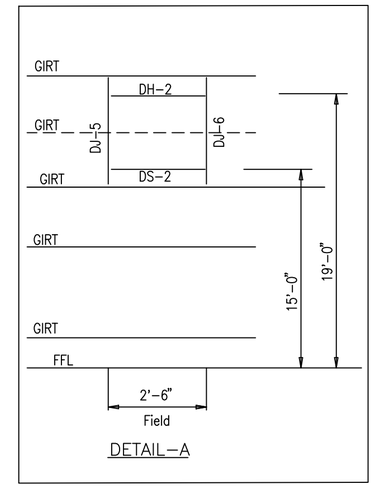
DETAIL–D



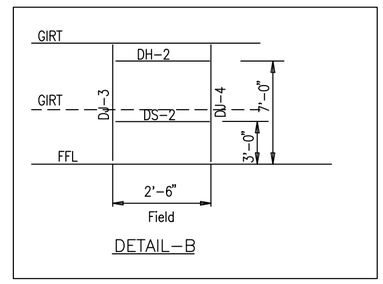
DETAIL–C



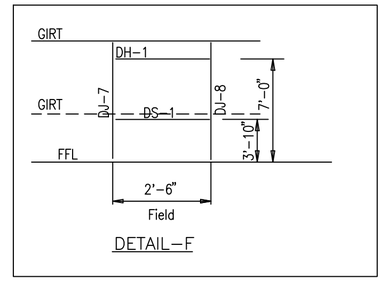
ENDWALL FRAMING: FRAME LINE 5



DETAIL–A



DETAIL–B



DETAIL–F

ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. SSX – SMP Cool White

[A] PANELS: 26 Ga. SSX – SMP–Patriot Red

BOLT TABLE FRAME LINE 5				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	4	A325	5/8"	1 1/2"

TRIM TABLE – THIS WALL ONLY FRAME LINE –5		
◇ID	PART	LENGTH
1	CT–102	13'–0"
2	FL–72	10'–3"
3	SF–5	8"
4	SF–2	20'–3"
5	SF–4	
6	SF–6	
7	WT–101	20'–3"
8	JT–101	
9	MT–116B	7'–6"
10	HT–101	3'–8"
11	MT–116B	3'–8"
12	JT–101	4'–4"
13	MT–116B	4'–4"
14	HT–101	2'–10"
15	MT–116B	2'–10"
16	MT–114	2'–10"
17	MT–116B	3'–6"
18	MT–116B	3'–6"

MEMBER TABLE FRAME LINE 5	
MARK	PART
EC–5	W10541
EC–6	W10541
EC–7	W10541
EC–8	W10541
DJ–3	10M25C14
DJ–4	10M25C14
DJ–5	10M25C14
DJ–6	10M25C14
DJ–7	10M25C14
DJ–8	10M25C14
DH–2	10M25C14
DS–2	10M25C14
G–10	10X25Z16
G–11	10X35Z12
G–12	10X35Z12
G–13	10X25Z16
G–14	10X35Z12
G–15	10X25Z16
G–16	10X35Z12
G–17	10X35Z14

CONNECTION PLATES FRAME LINE 5		
☐ID	MARK	PART
1	Z–1	4X25Z16
2	Z–2	4X25Z16
3	Z–3	4X25Z16
4	Z–4	4X25Z16
5	Z–5	4X25Z16

NOTE:
FEILD CUT IS REQUIRED FOR PANELS

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'–0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'–0, Super Seam 4'–0, Weather Lok–16 2'–8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot–rolled or built–up members must be pre–drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw–down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
2. Field cut or lap angles as required to fit.
3. Flange braces are marked by their length in decimal inches.
4. Outside flange of girt turns down unless noted.
5. Endwall girts and eave struts do not lap.
6. Field cut and self–tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1–1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1–3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
13. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw–down type roof for additional installation instructions.
14. Sub–jambs for overhead doors, if required, is not furnished by Metal Building Provider

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Final drawings for construction.

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ISSUE	DATE	DESCRIPTION	BY	CHK
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A1	09.08.22	FOR APPROVAL	BCB	PNR
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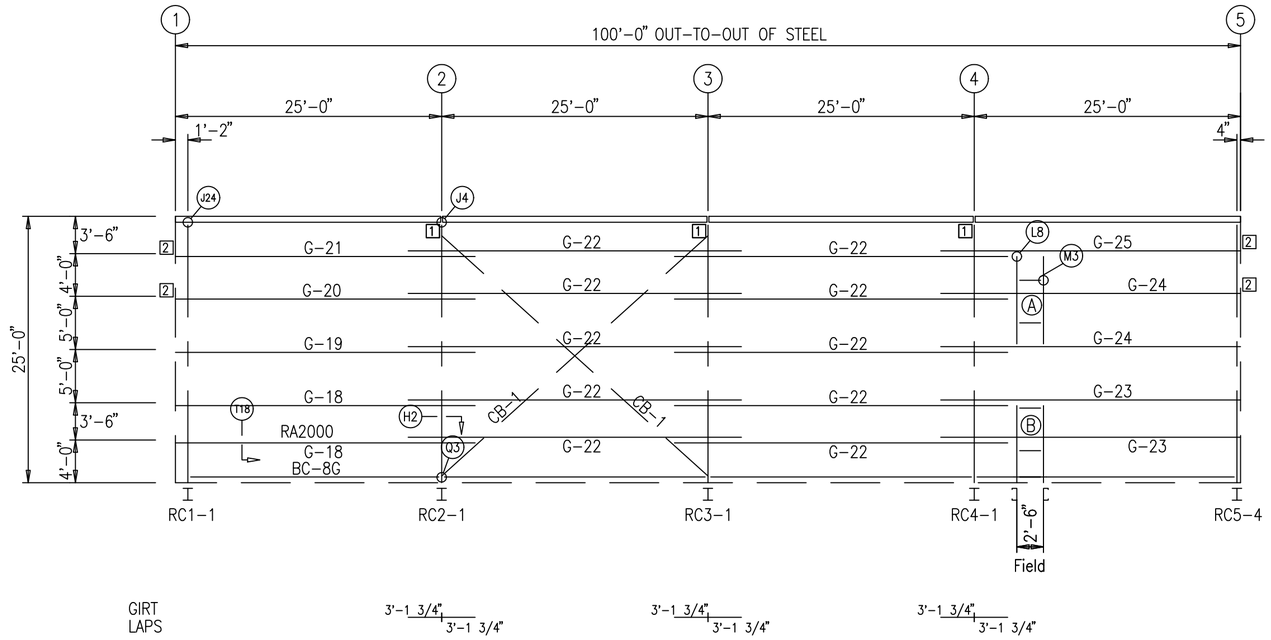
SHEET DESCRIPTION: ENDWALL ELEVATION						BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"	
CUSTOMER: CLASSIC AVIATION						CUSTOMER LOCATION: CORTEZ, CO 81321	
PROJECT REFERENCE: CLASSIC AVIATION							
JOBSITE LOCATION: CORTEZ, CO 81321						JOBSITE COUNTY: MONTEZUMA	
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780		DWG NO: E4	ISSUE: A2

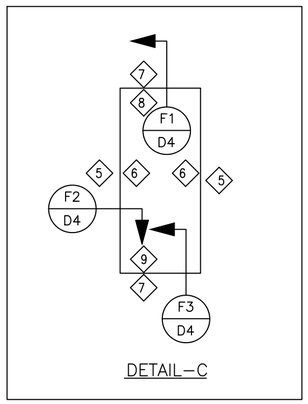
COLORADO LICENSED MUSTAPHA I. CHREIDIE 49784 PROFESSIONAL ENGINEER

12/1/2022

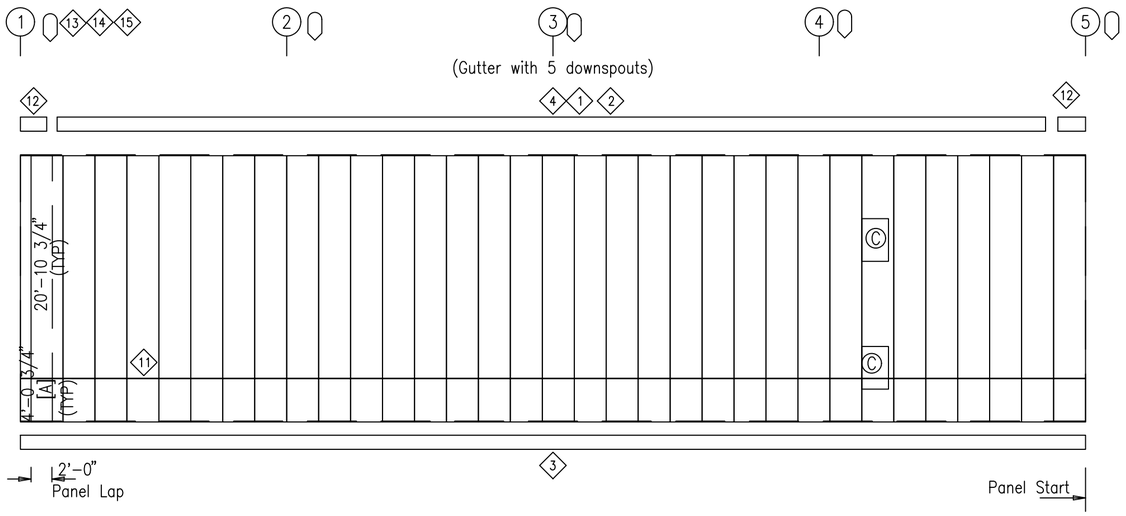
DOWNSPOUT LOCATIONS



SIDEWALL FRAMING: FRAME LINE H



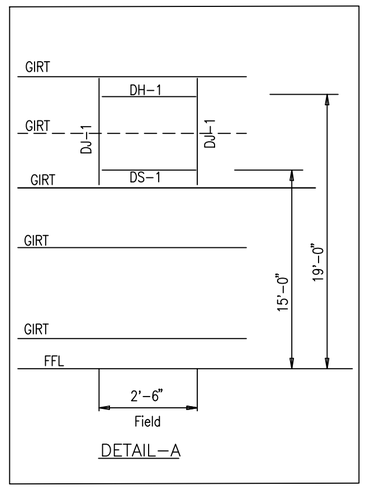
DETAIL–C



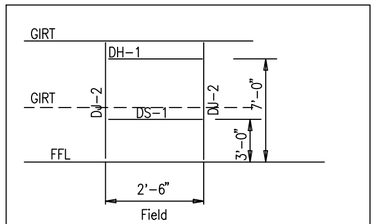
SIDEWALL SHEETING & TRIM: FRAME LINE H

PANELS: 26 Ga. SSX – SMP Cool White

[A] PANELS: 26 Ga. SSX –SMP– Patriot Red



DETAIL–A



DETAIL–B

TRIM TABLE – THIS WALL ONLY FRAME LINE – H		
◇ID	PART	LENGTH
1	GC-101	10'-3"
2	GS-120	
3	FL-72	10'-3"
4	SF-1	20'-3"
5	MT-116B	4'-4"
6	JT-101	4'-4"
7	MT-116B	2'-10"
8	HT-101	2'-10"
9	MT-114	2'-10"
11	WT-101	20'-3"
12	SF-3	
13	DS-101	12'-4"
14	DS-105	
15	DS-101	12'-8"

MEMBER TABLE FRAME LINE H	
MARK	PART
DJ-1	8M25C14
DJ-2	8M25C14
DH-1	8M25C14
DS-1	8M25C14
G-18	8X25Z16
G-19	8X25Z14
G-20	8X25Z14
G-21	8X25Z16
G-22	8X25Z16
G-23	8X25Z14
G-24	8X25Z12
G-25	8X25Z16
CB-1	1.00_ROD

CONNECTION PLATES FRAME LINE H	
□ID	MARK/PART
1	AK106
2	SC-5

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
2. Roof member screws are at 12" o.c. Eave end lap and peak screws are as shown.
3. Wall member screws are at 6" o.c. at the base member and 12" o.c. at all remaining members.
4. Roof stitch screws are located at each member with two between members (20" max. spacing).
5. Wall stitch screws are located at each member with one between members (20" max. spacing).
6. Skylight stitch screws are at 6" o.c.
7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'-0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'-0, Super Seam 4'-0, Weather Lok-16 2'-8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
15. Hot-rolled or built-up members must be pre-drilled before attaching members screws.
16. Metal shavings must be swept from the roof each day to avoid surface rusting.
17. Windows and louvers must be installed before sheeting the walls.
18. For clarity, tape sealant, closures, etc. may not be shown. Refer to the standing seam erection manual or standard pull out for screw-down type roof for additional installation instructions.

GENERAL FRAMING NOTES

1. Angles are marked by their length in feet and inches.
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6. Field cut and self-tap girts at walk doors.
7. Field slot girts for brace rods or cables.
8. Field locate windows and walk doors.
9. Field weld all splices at 14 gauge valley gutters.
10. Field bolt AK400 base clip to endwall columns:
    (2) 5/8" x 1-1/2" A325 bolts if (1) AK400 req'd
    (2) 5/8" x 1-3/4" A325 bolts if (2) AK400 req'd
11. Locate top of roof framed openings flush with the pan of the roof panel.
12. Some field drilling at framed openings may be required. Field drill 9/16" diameter holes.
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14. Sub-jambs for overhead doors, if required, is not furnished by Metal Building Provider

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CUSTOMER: CLASSIC AVIATION					CUSTOMER LOCATION: CORTEZ, CO 81321	
PROJECT REFERENCE: CLASSIC AVIATION						
JOBSITE LOCATION: CORTEZ, CO 81321					JOBSITE COUNTY: MONTEZUMA	
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: E5	ISSUE: A2

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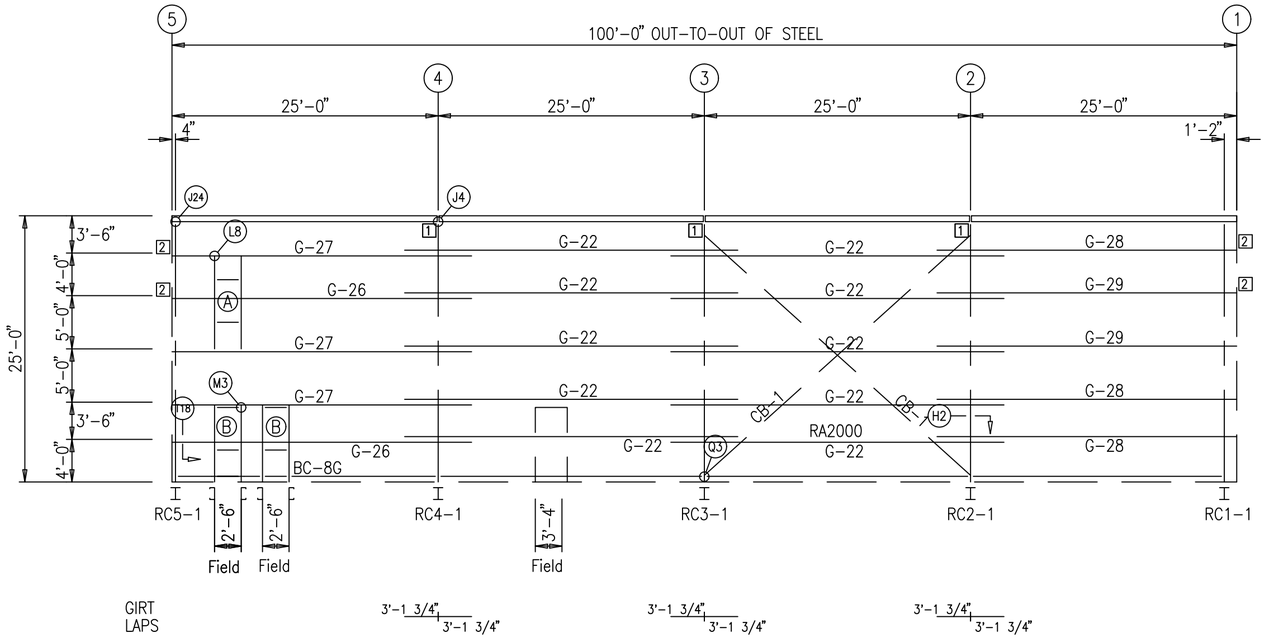
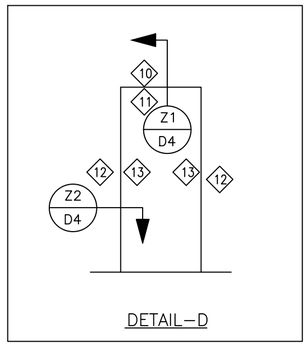
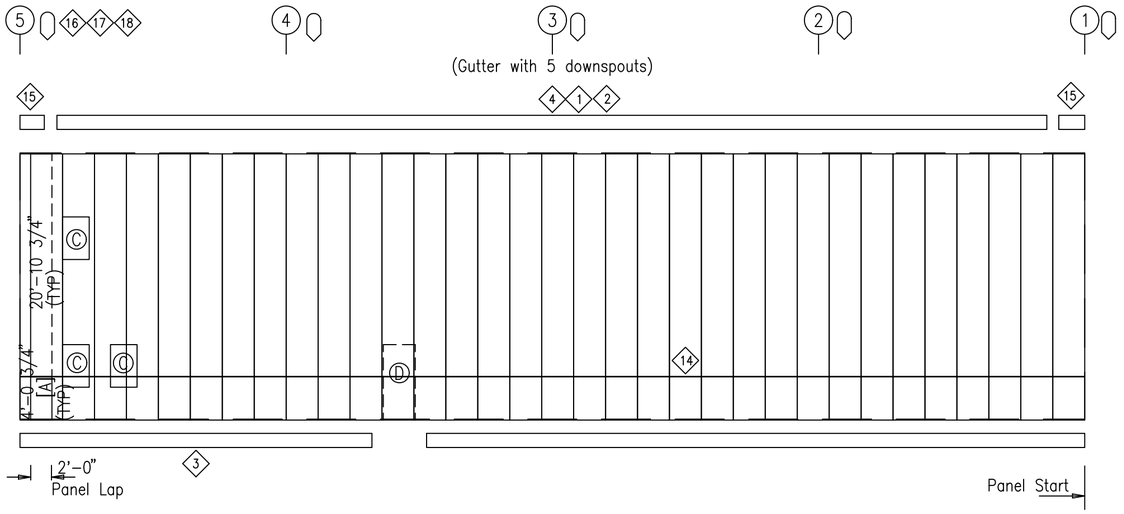
DOWNSPOUT LOCATIONS



SIDEWALL FRAMING: FRAME LINE A



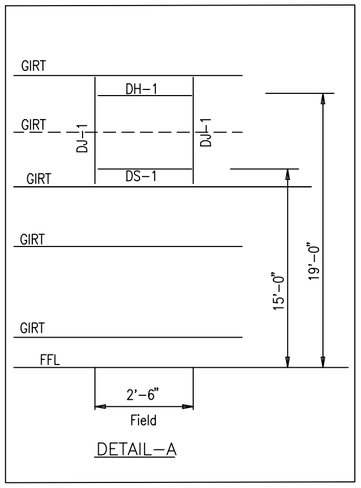
DETAIL–D



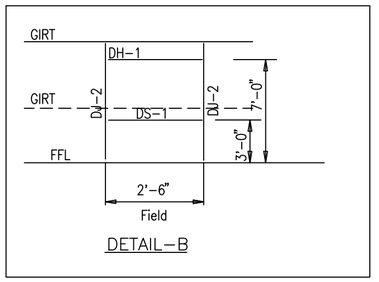
SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. SSX – SMP Cool White

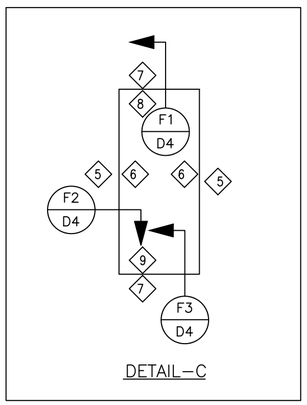
[A] PANELS: 26 Ga. SSX – SMP–Patriot Red



DETAIL–A



DETAIL–B



DETAIL–C

TRIM TABLE – THIS WALL ONLY FRAME LINE – A		
◇ID	PART	LENGTH
1	GC–101	10'–3"
2	GS–120	
3	FL–72	10'–3"
4	SF–1	20'–3"
5	MT–116B	4'–4"
6	JT–101	4'–4"
7	MT–116B	2'–10"
8	HT–101	2'–10"
9	MT–114	2'–10"
10	MT–116B	3'–8"
11	HT–101	3'–8"
12	MT–116B	7'–6"
13	JT–101	7'–6"
14	WT–101	20'–3"
15	SF–3	
16	DS–101	12'–4"
17	DS–105	
18	DS–101	12'–8"

MEMBER TABLE FRAME LINE A	
MARK	PART
DJ–1	8M25C14
DJ–2	8M25C14
DH–1	8M25C14
DS–1	8M25C14
G–22	8X25Z16
G–26	8X25Z16
G–27	8X25Z12
G–28	8X25Z16
G–29	8X25Z14
CB–1	1.00_ROD

CONNECTION PLATES FRAME LINE A	
□ID	MARK/PART
1	AK106
2	SC–5

GENERAL SHEETING & TRIM NOTES

1. Refer to erection drawings for rake angle locations.
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7. Start endwall panels at centerline of bldg. unless noted.
8. Gutter, rake, & eave trim lap 2". All other trims lap 1".
9. Field cut or lap panels as required to fit.
10. Field cut panels for all openings.
11. Pop rivet gutter counterflashing to wall panel on 3'–0 centers and caulk all laps.
12. Gutter support strap spacing: Super Span 3'–0, Super Seam 4'–0, Weather Lok–16 2'–8".
13. Corner and/or peak boxes are not furnished with special rake or gutter profiles. Field miter as req'd.
14. Downspout straps are located 6" from base and at every girt location.
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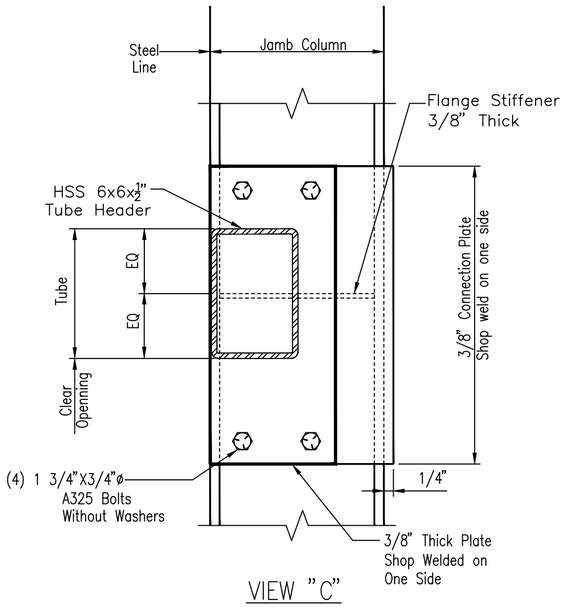
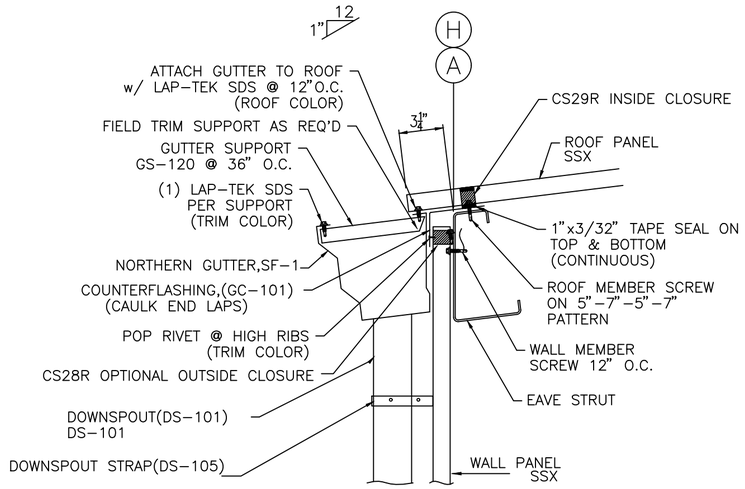
SHEET DESCRIPTION: SIDEWALL ELEVATION					BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"	
CUSTOMER: CLASSIC AVIATION					CUSTOMER LOCATION: CORTEZ, CO 81321	
PROJECT REFERENCE: CLASSIC AVIATION						
JOBSITE LOCATION: CORTEZ, CO 81321					JOBSITE COUNTY: MONTEZUMA	
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: E6	ISSUE: A2

COLORADO LICENSED
MUSTAPHA I. CHREIDIE
49784
PROFESSIONAL ENGINEER

12/1/2022

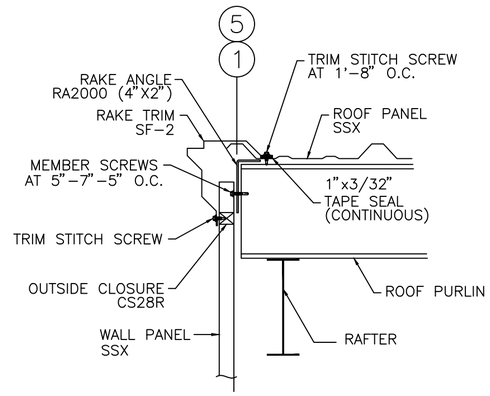


VIEW "C"

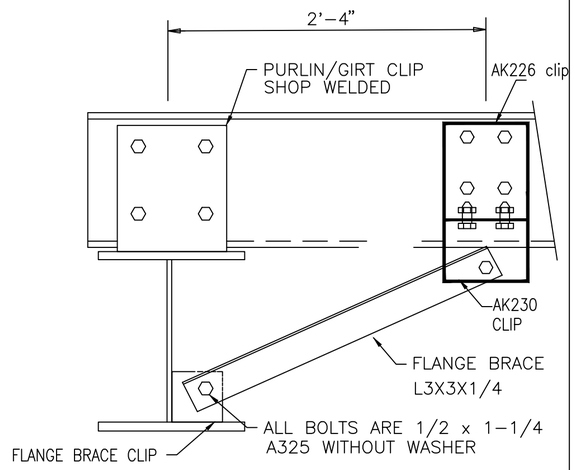


*LAP GUTTER 2" w/ (2) CONTINUOUS BEADS OF CAULK

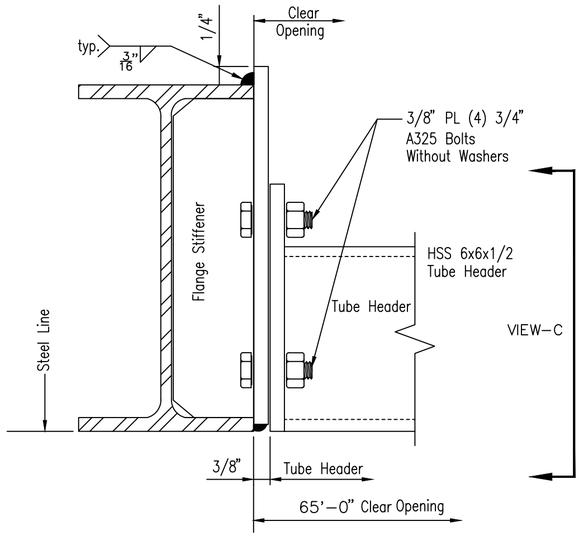
SECTION "B"



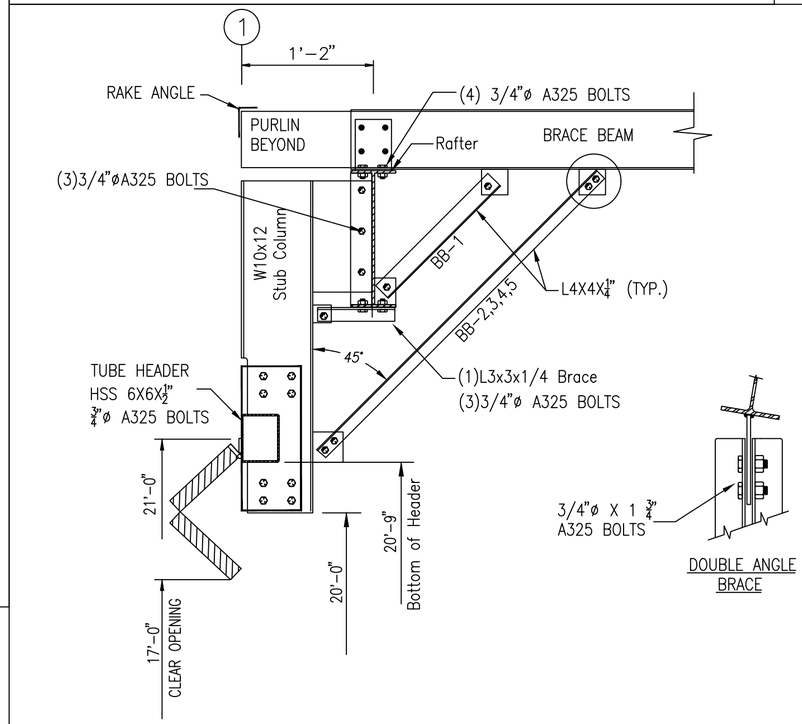
SECTION "A"



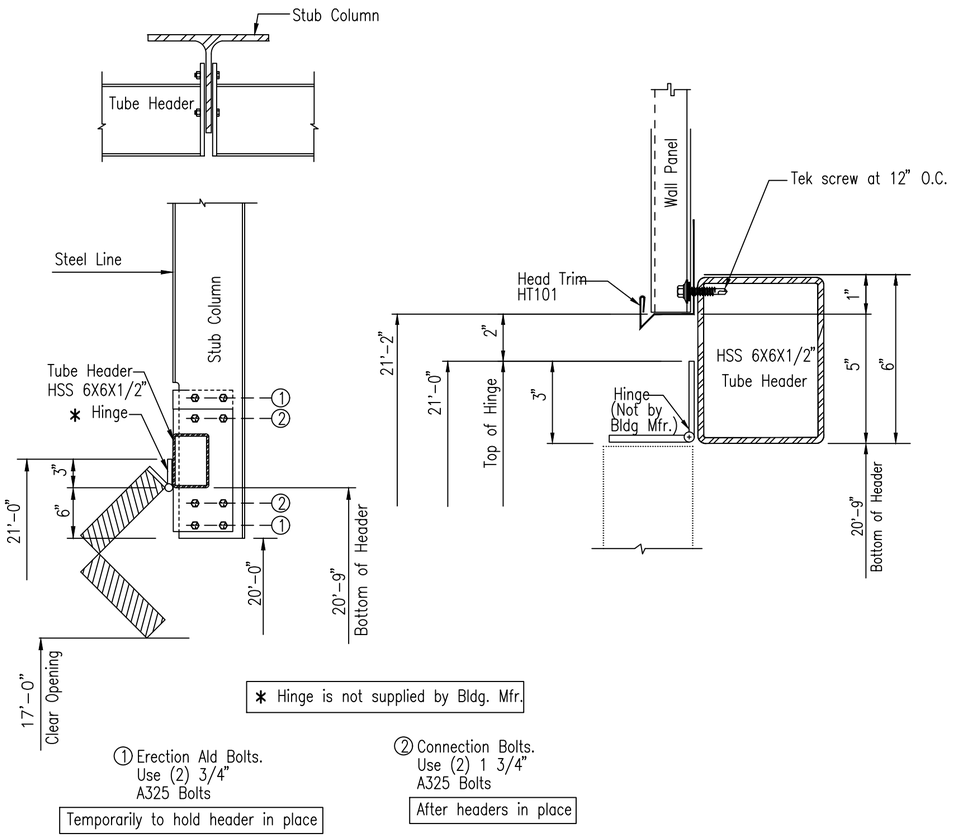
FLANGE BRACE DETAIL



SECTION "D"



DOOR BRACE BEAM CONNECTION

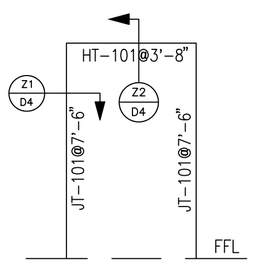


* Hinge is not supplied by Bldg. Mfr.

① Erection Ald Bolts. Use (2) 3/4" A325 Bolts
Temporarily to hold header in place

② Connection Bolts. Use (2) 1 3/4" A325 Bolts
After headers in place

BI-FOLD DOOR STUB COLUMN TO TUBE HEADER CONNECTION



(5) 3070 WALK DOOR TRIM KIT FIELD LOCATED

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- ☐ FOR CONSTRUCTION PERMIT: These drawings, being for permit,are by definition not final. Only drawings issued "For Erection installation" can be considered complete.
- ☐ FOR ERECTOR INSTALLATION: Final drawings for construction.

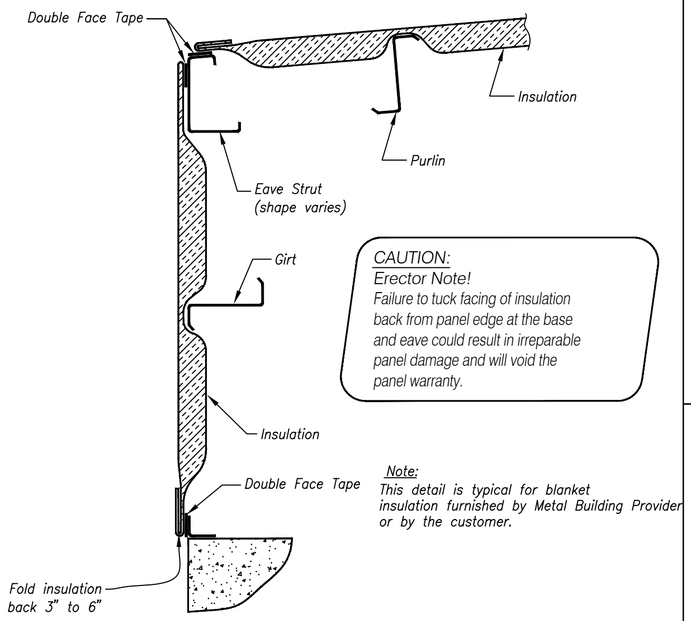
METALBUILDING OUTLET CORP.
7651 SHAFFER PARKWAY LITTLETON, CO 80127

ISSUE	DATE	DESCRIPTION	BY	CHK
0	07.01.22	FOR CONSTRUCTION PERMIT	KXJ	PNR
1	07.19.22	REV. FOR CONSTRUCTION PERMIT	BCB	PNR
A1	09.08.22	FOR APPROVAL	BCB	PNR
A2	11.23.22	REV.FOR APPROVAL	SXS	PNR

SHEET DESCRIPTION: SECTION DETAILS					BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"	
CUSTOMER: CLASSIC AVIATION					CUSTOMER LOCATION: CORTEZ, CO 81321	
PROJECT REFERENCE: CLASSIC AVIATION						
JOBSITE LOCATION: CORTEZ, CO 81321					JOBSITE COUNTY: MONTEZUMA	
DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: E7	ISSUE: A2

COLORADO LICENSED MUSTAPHA I. CHREIEIE 49784 PROFESSIONAL ENGINEER

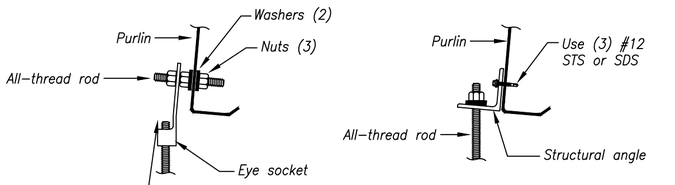
12/1/2022



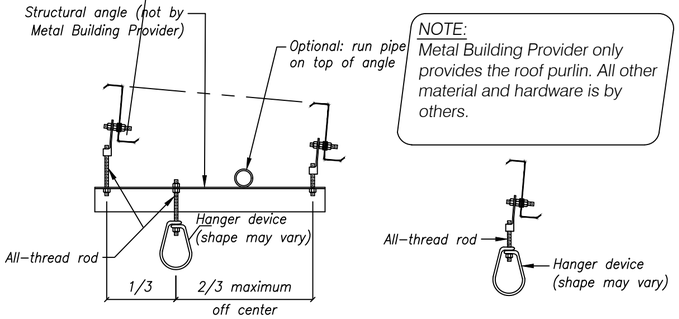
CAUTION:
Erector Note!
Failure to tuck facing of insulation back from panel edge at the base and eave could result in irreparable panel damage and will void the panel warranty.

Note:
This detail is typical for blanket insulation furnished by Metal Building Provider or by the customer.

STANDARD DETAIL @ BLANKET INSULATION



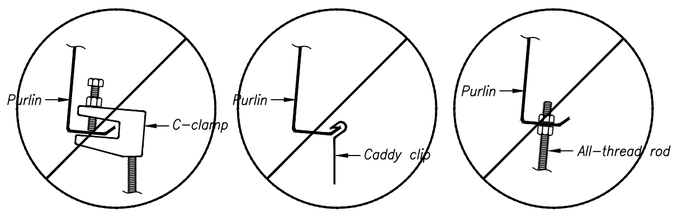
ACCEPTABLE CONNECTION METHODS



NOTE:
Metal Building Provider only provides the roof purlin. All other material and hardware is by others.

TRAPEZE HANGER ~ 4" MAX. PIPE

HANGER ~ 2-1/2" MAX. PIPE

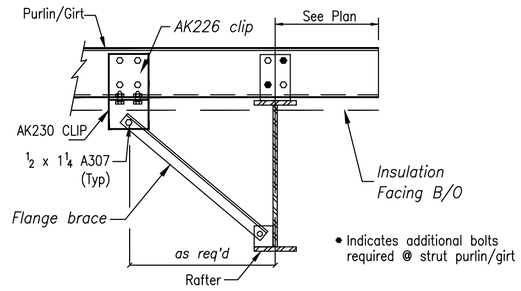


Flange C-Clamp is not an acceptable connection

Caddy clip attached to lip is not an acceptable connection

Connection through the flange is not acceptable

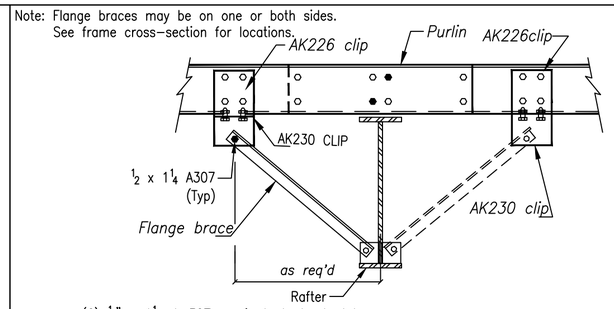
ACCEPTABLE CONNECTIONS FOR ALL COLLATERAL LOADS FOR HANGER ATTACHMENT



(6) 1/2" x 1 1/4 A–307 required at standard laps.
(8) 1/2" x 1 1/4 Required at strut members.
(A strut member is a purlin or girt at the bracing points.
The struts continue across the length of the building.)
See E–drawing for exceptions to size and quantity of bolts.

END DETAIL

Note: Flange braces may be on one or both sides.
See frame cross–section for locations.



(6) 1/2" x 1 1/4 A–307 required at standard laps.
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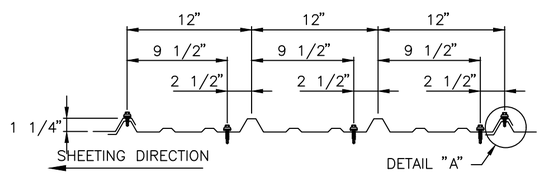
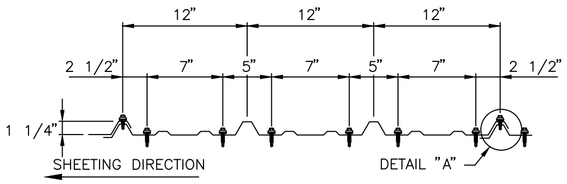
(A strut member is a purlin or girt at the bracing points.
The struts continue across the length of the building.)
See E–drawing for exceptions to size and quantity of bolts.

LAP DETAIL

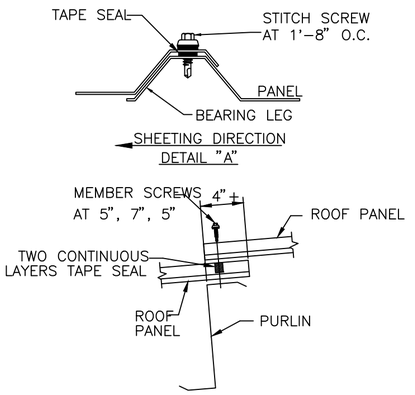


ALL ROOF MEMBERS EXCEPT AS NOTED BELOW



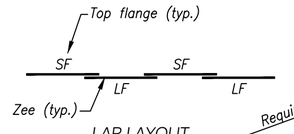
AT EAVE STRUT, PANEL END LAP, AND PEAK PURLIN

FASTENER LOCATION FOR "SUPER SPAN X" ROOF PANEL

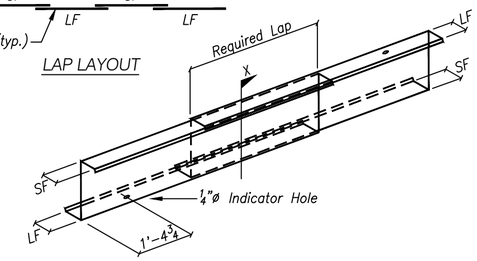


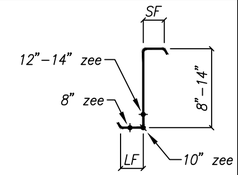
SECTION THRU PANEL END LAPS

Notes:
1. Rotate each zee as required to alternate between long and short flanges.
2. The 1/4"ø hole 1'–4 3/4 from each end is located nearest the long flange.
3. Lap connection bolts are omitted for clarity.

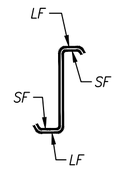


LAP LAYOUT





1/4" INDICATOR HOLE LOCATION

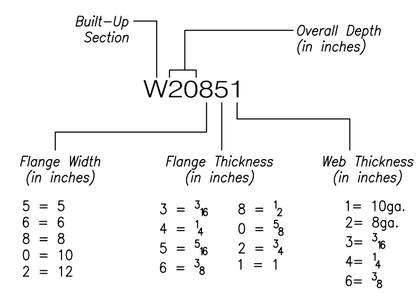


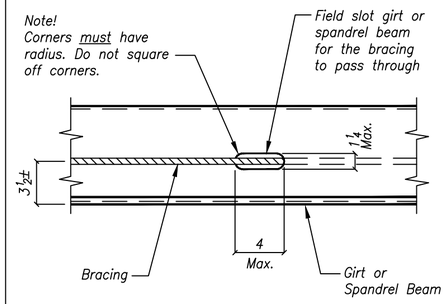
SECTION "X"

LF = Long Flange
SF = Short Flange

ZEE LAP DETAIL @ THE LONG AND SHORT FLANGES

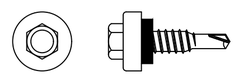
BUILT-UP SECTION LEGEND





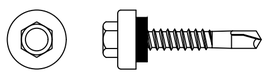
DETAIL @ FIELD LOCATED BRACING SLOT

Description: 1/4–14 x 7/8 Hex Head Undercut #1 Point Self–Drilling Lap Long–Life Zinc Die Cast Head (#14 x 7/8 Long–Life Lap–Tek S.D.S.)
Seating Torque: 30 to 60 in–lbs
Recommended Driving Tool: 1800 RPM electric screw gun with depth sensing nosepiece to prevent overdriving and stripout
Suggested Pre–Drill: None

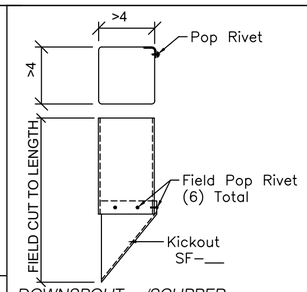


Actual Size

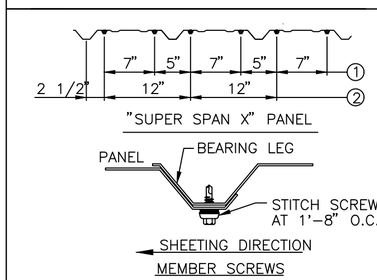
Description: 12–14 x 1 1/4 Hex Head Undercut #3 Long Pilot Point Self–Drilling Long–Life Zinc Die Cast Head (#12 x 1 1/4 Long–Life S.D.S.)
Seating Torque: 30 to 60 in–lbs
Recommended Driving Tool: 1800 RPM electric screw gun with depth sensing nosepiece to prevent overdriving and stripout
Suggested Pre–Drill: None



Actual Size



DOWNSPOUT w/SCUPPER
Field Install Scupper If Req'd



① = AT BASE, RAKE, EAVE, AND MID SPAN END LAPS

② = AT INTERMEDIATE MEMBER, AND AT OPTIONAL LINER PANEL

FASTENER LOCATION FOR WALL PANEL (SUPER SPAN X)

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  Final drawings for construction.

METAL BUILDING OUTLET CORP.
7651 SHAFFER PARKWAY LITTLETON, CO 80127

The Engineer whose seal and signature appear on these documents represent Whirlwind Steel Buildings, Inc., and is not the Engineer of Record for the overall project. The Engineer's responsibility is limited to material designed and manufactured by Metal Building Provider, and excludes parts such as doors, windows, foundation design, and erection of the building.

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ISSUE	DATE	DESCRIPTION	BY	CHK
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SHEET DESCRIPTION: STANDARD DETAIL PAGE	BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"
CUSTOMER: CLASSIC AVIATION	CUSTOMER LOCATION: CORTEZ, CO 81321
PROJECT REFERENCE: CLASSIC AVIATION	
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DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
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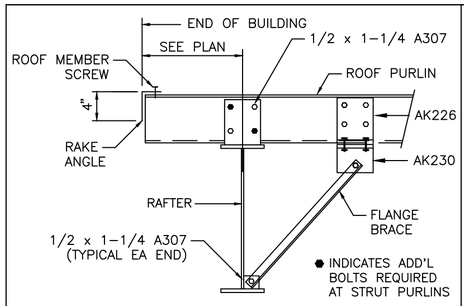
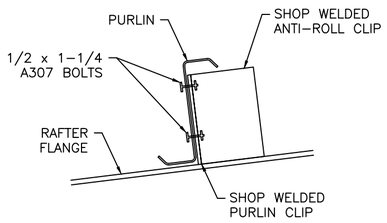


(2) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(4) 1/2 x 1-1/4 A307 REQUIRED AT STRUT MEMBERS

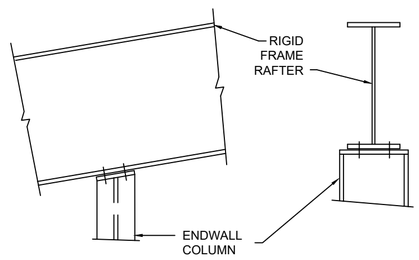
A STRUT PURLIN IS A PURLIN LOCATED AT THE BRACE POINTS. SEE PLANS FOR EXCEPTION TO SIZE & QTY OF BOLTS.

A10 ROOF PURLIN CONNECTION AT MAIN FRAME ENDWALL



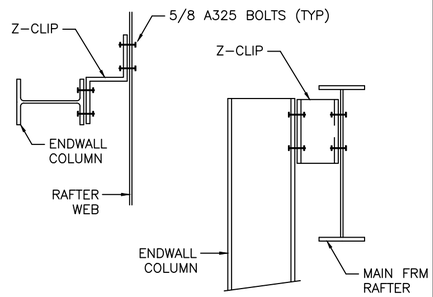
SOME PURLIN CLIPS WILL HAVE AN ADDITIONAL ANTI-ROLL CLIP ATTACHED. THE QUANTITY AND SPACING OF THESE CLIPS IS DETERMINED BY THE DESIGN FOR EACH SPECIFIC BUILDING.

ANTI DETAIL AT ANTI-ROLL CLIP

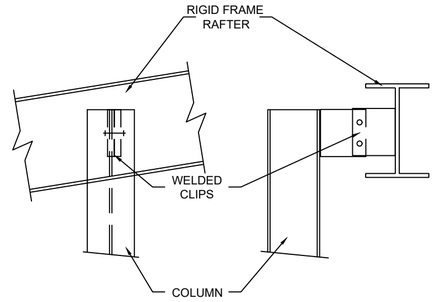


B20 COLUMN TO RIGID FRAME RAFTER

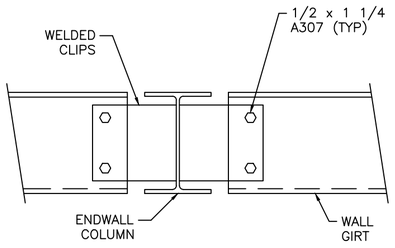
SEE PLANS FOR CLIP PART MARK AND REQUIRED BOLT LENGTH



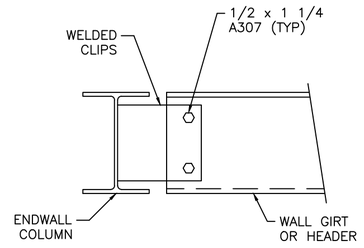
B34 I-SHAPED ENDWALL COLUMN TO MAIN FRAME END RAFTER CONNECTION



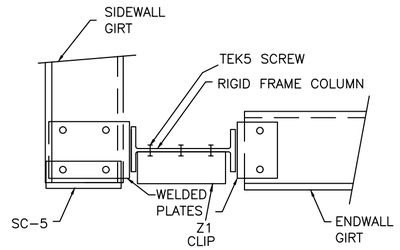
B35 R/W COLUMN TO RIGID FRAME RAFTER



C6 ENDWALL GIRT TO COLUMN

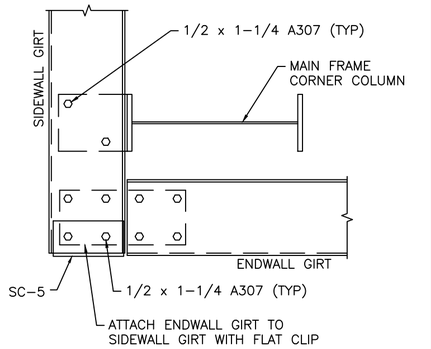


C15 GIRT/HEADER TO COLUMN

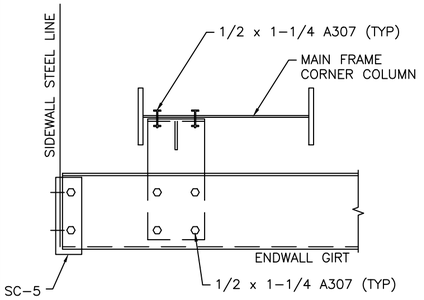


D15 CORNER COLUMN TO WALL GIRT

SEE PLANS FOR CLIP MARK

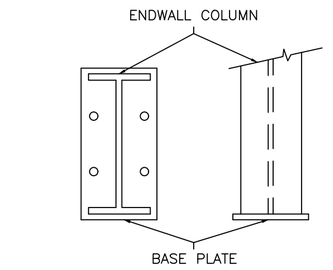


D16 GIRT CONNECTION AT CORNER SW & EW GIRTS AT SAME ELEVATION



SEE ERECTION PLANS FOR CLIP PART MARKS

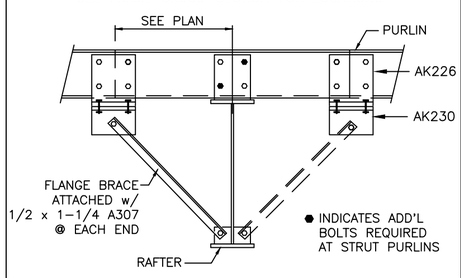
D27 GIRT CONNECTION AT CORNER SW & EW GIRTS AT DIFFERENT ELEVS.



SEE BOLT SETTING PLAN FOR ACTUAL BOLT QTY.

E3 BASE PLATE FOR ENDWALL COLUMN

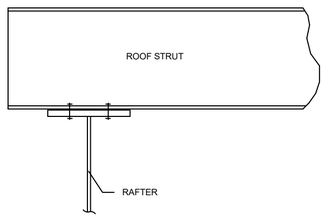
FLANGE BRACES MAY BE ON ONE OR BOTH SIDES
SEE FRAME CROSS-SECTION FOR LOCATIONS



(6) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(8) 1/2 x 1-1/4 REQUIRED AT STRUT MEMBERS

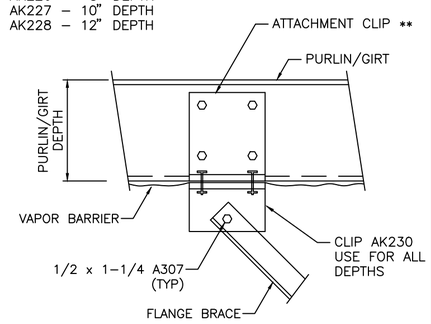
A STRUT MEMBER IS A PURLIN LOCATED AT THE BRACE POINTS. SEE PLANS FOR EXCEPTIONS TO SIZE & QTY OF BOLTS.

G2 ROOF PURLIN TO INTERIOR FRAME RAFTER



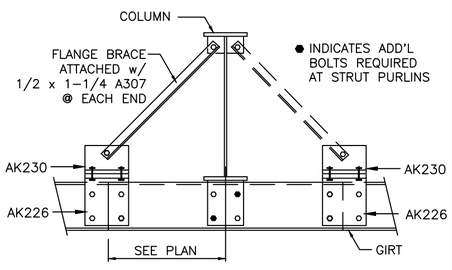
G17 BYPASS ROOF STRUT TO RIGID FRAME RAFTER

** CLIP PART MARK
AK226 – 8" DEPTH
AK227 – 10" DEPTH
AK228 – 12" DEPTH



G26 FLANGE BRACE TO PURLIN/GIRT CLIPS OPTIONAL USE WITH VAPOR BARRIER

FLANGE BRACES MAY BE ON ONE OR BOTH SIDES
SEE FRAME CROSS-SECTION FOR LOCATIONS



(6) 1/2 x 1-1/4 A307 REQUIRED AT STANDARD LAPS
(8) 1/2 x 1-1/4 A307 REQUIRED AT STRUT MEMBERS

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H2 WALL GIRT TO FRAME COLUMN

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METALBUILDING OUTLET CORP.
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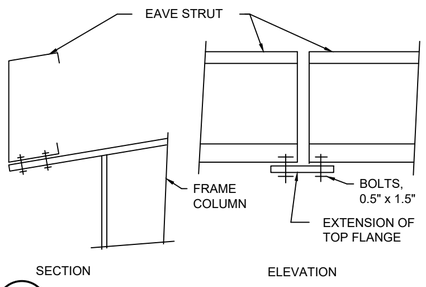
A2	11.23.22	REV.FOR APPROVAL	SXS	PNR

SHEET DESCRIPTION: STANDARD DETAIL PAGE
BLDG. SIZE: 100'-0" X 100'-0" X 25'-0"
CUSTOMER: CLASSIC AVIATION
CUSTOMER LOCATION: CORTEZ, CO 81321
PROJECT REFERENCE: CLASSIC AVIATION
JOBSITE LOCATION: CORTEZ, CO 81321
JOBSITE COUNTY: MONTEZUMA

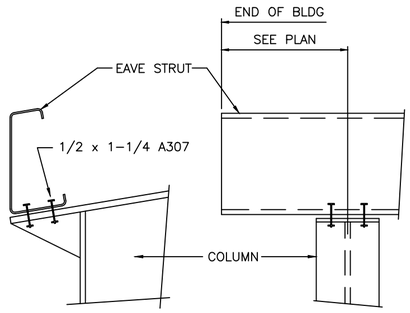
DWN:	CHK:	DATE:	ENG:	JOB NO:	DWG NO:	ISSUE:
SXS	PNR	11.23.22	AJF	9480-28780	D2	A2

COLORADO LICENSED MUSTAPHA I. CHREIDE 49784 PROFESSIONAL ENGINEER

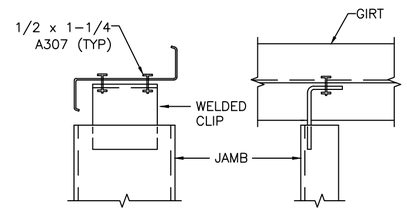
12/1/2022



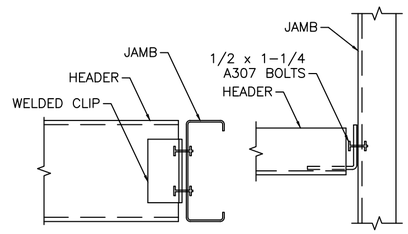
J4 EAVE STRUT TO RIGID FRAME



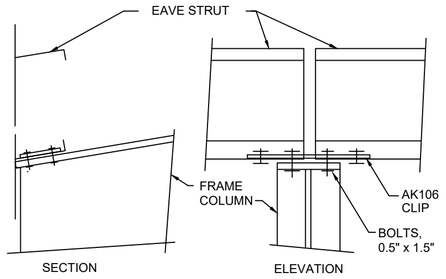
J24 EAVE STRUT TO RIGID FRAME



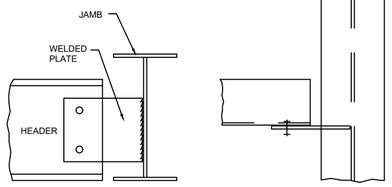
L8 DOOR JAMB TO WALL GIRT



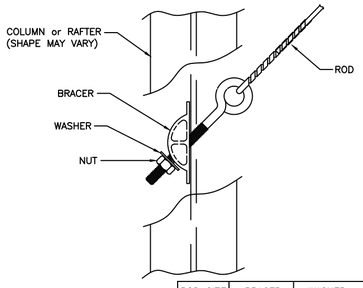
M3 HEADER TO CEE JAMB



J3 EAVE STRUT TO RIGID FRAME

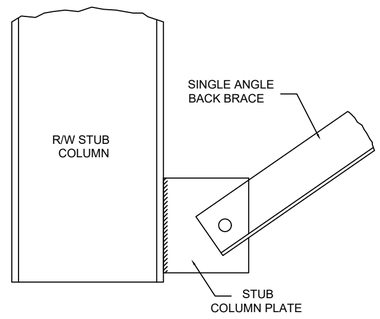


M13 HEADER TO R/W JAMB

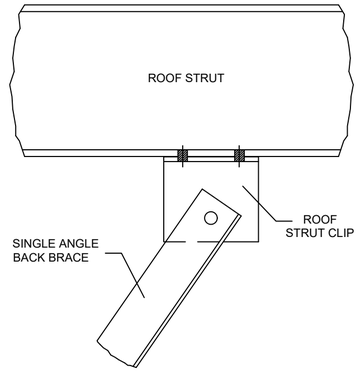


ROD SIZE	BRACER	WASHER	NUT
1/2"	BRACER #1	F844 1/2"	A563 1/2"
5/8"	BRACER #1	F844 5/8"	A563 5/8"
3/4"	BRACER #2	F844 3/4"	A563 3/4"
1"	BRACER #3	F844 1"	A563 1"
1 1/4"	BRACER #4	F844 1 1/4"	A563 1 1/4"

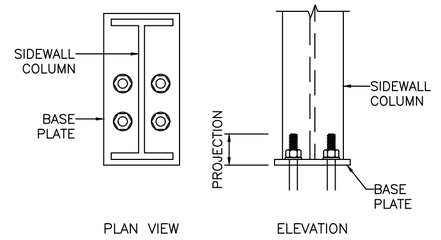
Q3 DIAGONAL ROD BRACING INSTALLATION



Q7 BACK BRACE TO R/W STUB COLUMN

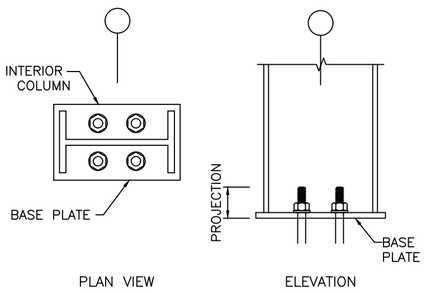


Q14 BACK BRACE TO ROOF STRUT



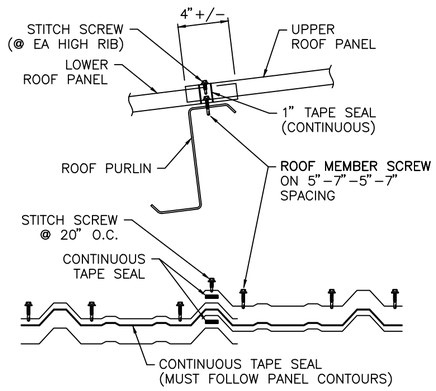
- ALL ANCHOR RODS (BY OTHERS) TO HAVE NUTS AND FLAT WASHERS.
- SEE BOLT SETTING PLAN FOR ACTUAL BOLT QTY.

R2 ANCHOR RODS AT SIDEWALL COLUMN

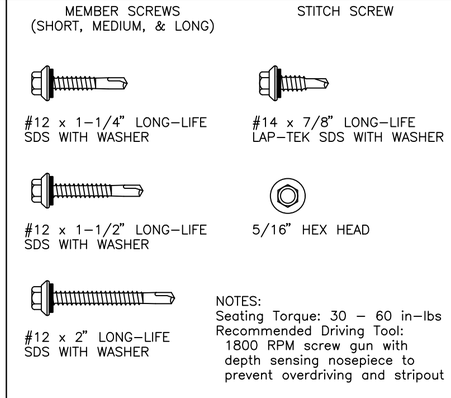


- ALL ANCHOR RODS (BY OTHERS) TO HAVE NUTS AND FLAT WASHERS.
- SEE BOLT SETTING PLAN FOR ACTUAL BOLT QTY.

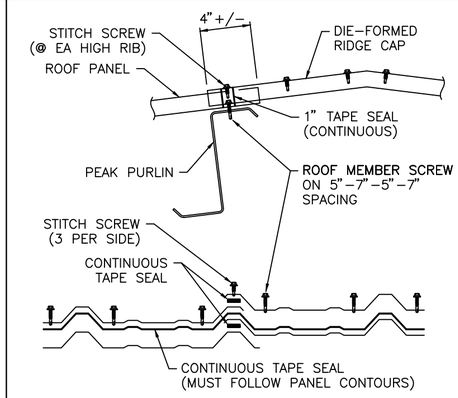
S2 ANCHOR RODS AT INTERIOR COLUMN



SCREW_2 TYPICAL ROOF PANEL ENDLAP SUPER SPAN X ROOF



SCREW_4 STANDARD FASTENERS LONG-LIFE SELF-DRILLING



SCREW_8 TYPICAL DIE-FORMED RIDGECAP ENDLAP - SUPER SPAN X ROOF

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DWN: SXS	CHK: PNR	DATE: 11.23.22	ENG: AJF	JOB NO: 9480-28780	DWG NO: D3	ISSUE: A2

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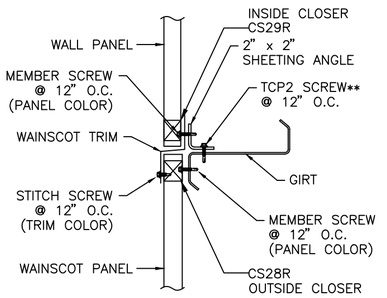
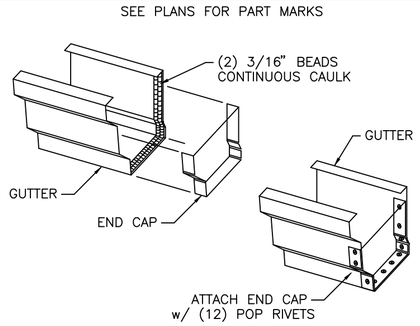
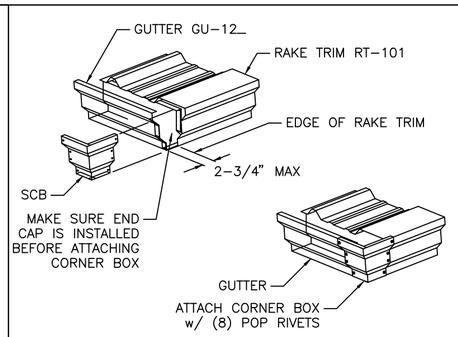


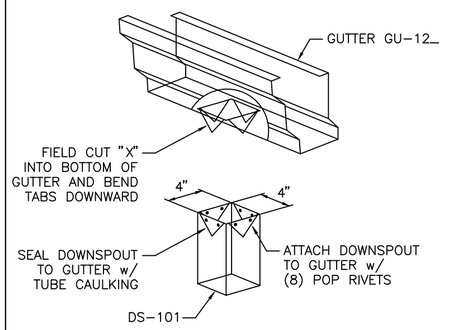
T18 SECTION THRU WAINSCOT TRANSITION PANELS NOT LAPPED



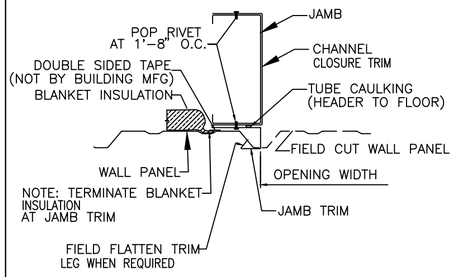
TRIM_4 GUTTER END CAP ATTACHMENT



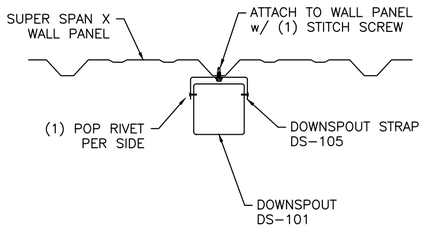
TRIM_5 CORNER BOX ATTACHMENT



TRIM_6 DOWNSPOUT ATTACHMENT @ GUTTER

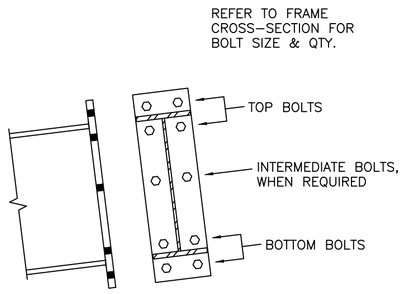


F2 JAMB TRIM INSTALLATION (SUPER SPAN X)

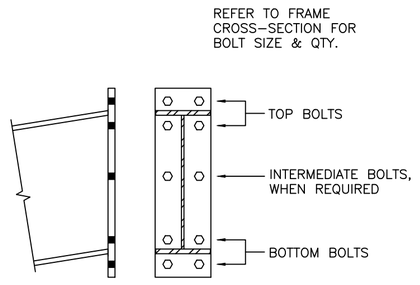


ATTACH ONE DS-105 STRAP AT THE BASE OF THE DOWNSPOUT THEN APPROXIMATELY 5'-0" CENTERS (8'-0" MAX.)

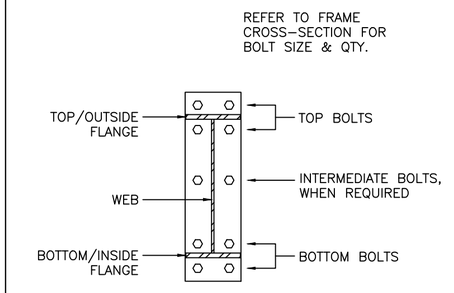
TRIM_7 DOWNSPOUT STRAP ATTACHMENT



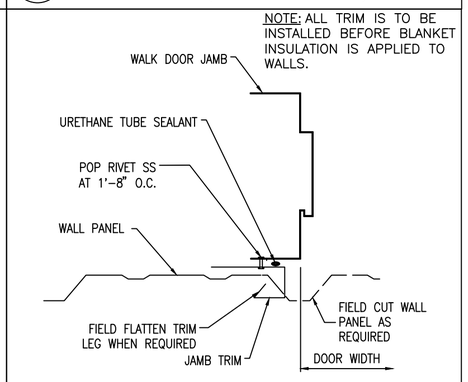
U1 BOLTED END PLATE RAFTER SPLICE



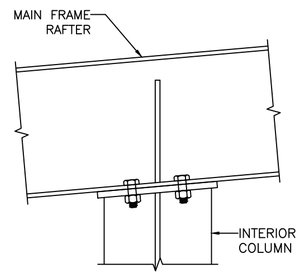
U2 BOLTED END PLATE CONNECTION AT BUILDING PEAK



U3 BOLTS FOR RAFTER TO COLUMN CONNECTION

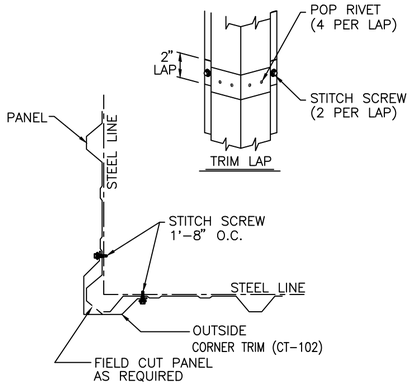


Z1 WALK DOOR STD JAMB TRIM (SUPER SPAN X)

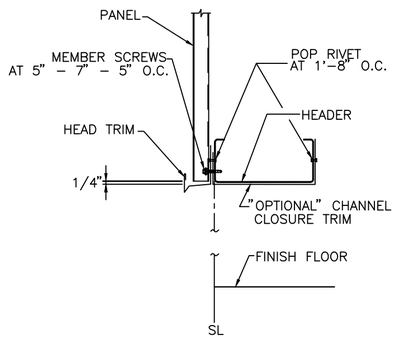


- SHAPE OF RAFTER MAY VARY. SEE FRAME CROSS-SECTION FOR ACTUAL PROFILE AND DIMENSIONS.
- SEE CROSS-SECTION FOR CONN. BOLT REQUIREMENTS.

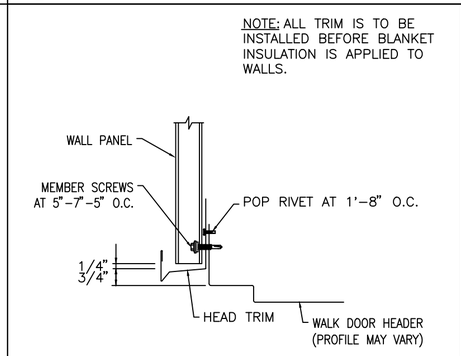
V2 INTERIOR COLUMN TO RAFTER CONN.



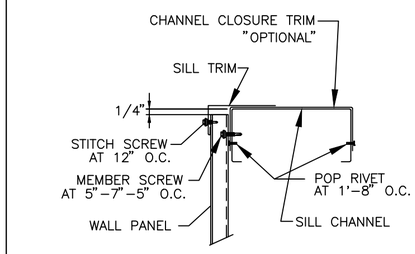
C1 CORNER TRIM INSTALLATION



F1 HEAD TRIM INSTALLATION (SUPER SPAN X)



Z2 WALK DOOR STD HEAD TRIM (SUPER SPAN X)



F3 SILL TRIM INSTALLATION (SUPER SPAN X)

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