METAL BUILDING OUTLET CORP.

CUSTOMER END USER RESPONSIBILITIES

1. The CUSTOMER / END USER, hereafter referred to as the "CUSTOMER", obtains and pays for all building permits, licenses, public assessments, paving or utility pro rata, utility connections, occupancy fees and other fees required by any governmental authority or utility in connection with the work provided for in the Contract Documents. The CUSTOMER provides at his expense all plans and specifications required to obtain a building permit. It is the CUSTOMER'S responsibility to ensure that all plans and specifications comply with the applicable requirements of any governing building authorities.
2. The CUSTOMER is responsible for identifying all applicable building codes, zoning codes, or other regulations applicable to the Construction Project, including the metal building system in order to insure that Building Supplier's plans comply with the applicable requirements of any governing building authorities and to obtain appropriate approvals and secure necessary permits from City, County, State, OR Federal Agencies as required.
3. It is the responsibility of the CUSTOMER to interpret all aspects of the END USER'S specifications and incorporate the appropriate specifications, design criteria, and design loads into the Order Documents submitted to Building Supplier.
4. CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with Building Supplier's "For Construction" drawings only. Temporary supports such as guys, braces, false work, cribbing or other elements required for the erection operation shall be determined, furnished and installed by the ERECTOR. No items should be purchased from a preliminary set of drawing, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (Section 7, Code of Standard Practice for Steel Buildings – AISC 15th Edition.)
5. Building Supplier's standard specifications apply unless stipulated otherwise in the Contract Documents. Building Supplier design, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary not withstanding. It is understood by both parties that the CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications. In case of discrepancies between Building Supplier's structural steel plans and plans for other trades, Building Supplier's plans shall govern. (Section 3, Code of Standard Practice for Steel Buildings –Buildings, AISC 15th Edition).
6. It is the responsibility of Building Supplier, through their Engineer, to design the metal building system to meet the specifications including the design criteria and design loads incorporated by the CONTRACTOR into the Order Documents. Building Supplier is not responsible for making an independent determination of any local codes or any other requirements not part of the Order Documents.
7. Building Supplier is responsible only for the structural design of the metal building system. The Building Supplier's Engineer is not the Design Professional or Engineer of Record for the Construction Project. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that the Building Supplier or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components.
8. Building Supplier is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete. Unless otherwise provided in the Order Documents, Building Supplier does not and is not responsible for the design, material and construction of the foundation or foundation embedment. The CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site. It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Chapter IV Section 3.2.2 Metal Building Systems Manual 2018 Edition).
9. Building Supplier's standard specifications apply unless stipulated otherwise in the Contract Documents. Building Supplier's design, quality criteria, standards, practice, methods and tolerances shall govern the work any other interpretations to the contrary notwithstanding. It is understood by both parties that the CUSTOMER is responsible for clarifications of inclusions or exclusions from the Architectural plans.
10. In case of discrepancies between Building Supplier's structural steel plans and plans for other trades, Building Supplier shall govern ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC Manual, Section 3.3)
11. The CUSTOMER is responsible for overall project coordination. All interface, compatibility and design considerations concerning any materials not furnished by Building Supplier and Building Supplier's steel system are to be considered and coordinated by the CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or Building Supplier's assumptions will govern.
12. Anchor bolts and foundation bolts are designed, furnished, and set by the CUSTOMER in accordance with an approved drawing. Dimensional accuracy shall satisfy the requirements of Section 7.5.1 of "Code of Standard Practice for Steel Buildings and Bridges" in the AISC 15th edition Manual.
13. All other embedded items or connection materials between the structural steel and the work of other trades are located and set by the CUSTOMER in accordance with approved location on erection drawings. Accuracy of these items must satisfy the erection tolerance requirements.
14. Building Supplier does not investigate the influence of the metal building system on existing buildings or structures. The CUSTOMER assures that such buildings and structures are adequate to resist snow drifts, wind loads, or other conditions as a result of the presence of the metal building systems.

APPROVAL NOTES

1. Approval of Building Supplier's drawings and/or calculations indicates that Building Supplier has correctly interpreted the contract requirements. This approval constitutes the CUSTOMER'S acceptance of the Building Supplier's design, concepts, assumptions, and loadings. (Section 4, Code of Practice for Steel Buildings, AISC 15th Edition and MBSM 3.3.3).
2. Failure to respond to clouded areas and areas to verify may result in additional costs and/or schedule delays for which Building Supplier will not be responsible.
3. Any changes made after the CUSTOMER has signed and returned the approval drawings and/or calculations and the project is released for production shall be billed to the CUSTOMER including material, engineering, and other cost. An additional fee may be charged if the project must be moved from the engineering and/or the production/drafting schedule.
4. It is the responsibility of the CUSTOMER to field verify all existing conditions prior to fabrication.
5. It is imperative that any changes to these drawings:
   - 5.1. Be made in contrasting ink.
   - 5.2. Be legible and unambiguous.
   - 5.3. Have all instances of changes clearly indicated.
6. A dated signature, in the designated areas, is required on all pages. The signature must be from the person authorized on the contract or a person authorized, in writing, by the CUSTOMER.
7. Building Supplier reserves the right to resubmit drawings with extensive or complex changes required to avoid fabrication errors. This may impact the delivery schedule.
8. Any changes noted on the drawings not in conformance with the terms and requirements of the contract between Building Supplier and its CUSTOMER are not binding on Building Supplier unless subsequently specifically acknowledged and agreed to in writing by change order or separate documentation.
9. The CUSTOMER approves of all notes and conditions on the drawings and/or calculations by signing an Approval Drawing Waiver Form.

GENERAL NOTES

1. Wall and liner panels are an integral part of the structural system. Unauthorized removal of panels or cutting panels for framed openings not shown is prohibited.
2. Oil-canning, a perceived waviness inherent to light gauge metal, may exist. This condition does not affect the structural integrity or the finish of the panel, and therefore is not a cause for rejection.
3. The primer for all cold-formed structural framing members contain a "wax-type" lubricant to facilitate roll-forming. Hair-line crazing which may occur during forming operations is considered normal and is not a cause for rejection.
4. All other primed structural members are given one shop coat (1.0 mils) of standard red-oxide primer designed for short term field protection. This paint is not intended for long term exposure to the elements.
5. All bolts are 1/2" x 1-1/4" A307 except at bearing frame rafter splice, endwall column to rafter and main frame connections. Refer to drawings. Note: Washers are not supplied unless noted otherwise on drawing.
6. All high strength bolts are A325 unless specifically noted otherwise. Structural joints with A.S.T.M. A325 high strength bolts where indicated on the drawings are designed and considered to be in a Non-Slip Critical Category and therefore need only to be tightened to the snug tight condition. This condition should be attained when all surface in a joint are in firm contact and by using few impacts of an impact wrench or the full effort of a person using spud wrench. Hardened washers are not required unless otherwise on the drawings.
7. Any type of suspended or load inducing system(s) is prohibited if zero collateral and zero sprinkler loads are designated on the contract. This would include lights, duct work, piping, insulation types other than 3" standard duty fiberglass blanket insulation, etc.
8. Fabrication shall be in accordance with Building Supplier's standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3".

MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
Hot Rolled Steel Shapes (W, S, C & L)	A572 / A529	Fy = 50 KSI
Hot Rolled Steel Shapes (W)	A992	Fy = 50 KSI
Round Structural Tubing (HSS)	A500	Fy = 42 KSI
Square / Rect. Structural Tubing	A500	Fy = 46 KSI
Structural Steel Web Plate	A572 / A1011	Fy = 55 KSI
Structural Steel Flange Plates / Bars	A529 / A572	Fy = 55 KSI
Cold Formed Light Gage	A653 / A1D11	Fy = 55 KSI
Roof and Wall Sheets	A792 / A653	Fy = 50, 80 KSI
Cable Brace	A475	Extra High Strength
Rod Brace	A36	Fy = 36 KSI
		MIN. TENSILE STRENGTH
Machine Bolts & Nuts	A307	Fu = 60 KSI
High Strength Bolts (1" diam. and less)	A325	Fu = 120 KSI
High Strength Bolts (>1" diam. to 1 1/2 diam.)	A325	Fu = 105 KSI
Anchor Bolts	A36 / A307 / F155 Gr. 36	Fu = 58-80 KSI

THE METAL BUILDING MANUFACTURER RESERVES THE RIGHT TO SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIAL.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED

DESIGN LOADS	
DESIGN CODE	IBC 21
RISK CATEGORY	II – Normal
ENCLOSURE	Enclosed
DEAD LOAD (psf)	2
COLLATERAL LOAD (psf)	3.00
WIND LOAD	
WIND SPEED (BASIC OR ULT. PER CODE)	115
WIND IMPORTANCE FACTOR (Iw)	1.00
WIND EXPOSURE	C
INTERNAL PRESSURE COEF., GCPi	0.18 / –0.18
LIVE LOAD	
PRIMARY FRAMING (psf)	20.00
TRIBUTARY AREA REDUCTION	No
SECONDARY FRAMING (psf)	20.00
SNOW LOAD	
GROUND SNOW LOAD, Pg (psf)	40
ROOF SNOW LOAD, Pf (psf)	28.00
SLOPED ROOF SNOW LOAD, Ps (psf)	Pf x Cs
SNOW EXPOSURE FACTOR, Ce	1.0000
SNOW IMPORTANCE FACTOR, Is	1.0000
THERMAL FACTOR, Ct	1.00
SLOPED FACTOR, Cs	1.0000
SEISMIC LOAD	
SEISMIC IMPORTANCE FACTOR, Ie	1.00
SEISMIC OCCUPANCY CATEGORY	II – Normal
SITE CLASS	d
MAPPED SPECTRAL RESPONSE ACCEL.	Ss = 0.570 S1 = 0.184
SPECTRAL RESPONSE COEFFICIENT	Sds = 0.511 Sd1 = 0.274
SEISMIC DESIGN CATEGORY	D
BASIC FORCE RESISTING SYSTEMS USED	STEEL ORDINARY MOMENT RESISTING FRAMES
	STEEL ORDINARY CONCENTRICALLY
	BRACED FRAMES
TOTAL DESIGN BASE SHEAR, V (kips)	TRANSVERSE = 8.28
	LONGITUDINAL = 8.24
RESPONSE MODIFICATION FACTORS, R	RIGID FRAMES = 3.25
	END WALL BRACING = 3.25
	SIDE WALL BRACING = 3.25
SEISMIC RESPONSE COEFFICIENT, Cs	RIGID FRAMES = 0.157
	E. W. X BRACING = 0.157
	S. W. X BRACING = 0.157
ANALYSIS PROCEDURE USED	EQUIV. LATERAL FORCE PROCEDURE
RAINFALL INTENSITY (inches /Hr)	I1 = 3.0100 I2 = 3.7300

FRAMING / PANELS AND TRIMS	
FRAMING COATING	
PRIMARY & SECONDARY	GRAY OXIDE
ROOF PANELS	
GA / PANEL TYPE	26 / PBR
PANEL COLOR	GALVALUME+
ROOF TRIM COLORS:	
GA / EAVE COLOR	26 / CHARCOAL GRAY
GA / GUTTER COLOR	26 / CHARCOAL GRAY
GA / GABLE COLOR	26 / CHARCOAL GRAY
WALL PANELS	
GA. / PANEL TYPE	26 / PBR
PANEL COLOR	ASH GRAY
WALL TRIM COLORS	
GA / CORNER COLOR	26 / ASH GRAY
GA / OPENING COLOR	26 / CHARCOAL GRAY
GA / DOWNSPOUT COLOR	26 / CHARCOAL GRAY
GA / BASE TRIM COLOR	26 / CHARCOAL GRAY
WAINSCOT PANELS	
GA / PANEL TYPE	26 / PBR
PANEL COLOR	CHARCOAL GRAY
WAINSCOT TRIM	
WAINSCOT TRIM COLOR	N/A
LINER ROOF PANELS	
GA / PANEL TYPE	/
PANEL COLOR	
LINER WALL PANELS	
GA / PANEL TYPE	/
PANEL COLOR	
LINER TRIM	
LINER TRIM COLOR	
HP DOOR PANELS	
GA / PANEL TYPE	26 / PBR
PANEL COLOR	ASH GRAY
HP DOOR TRIM	
HP DOOR TRIM COLOR	26 / CHARCOAL GRAY
HP DOOR WAINSCOT PANELS	
GA / PANEL TYPE	N/A
PANEL COLOR	N/A
SOFFIT TRIM	
SOFFIT TRIM COLOR	
FASCIA PANELS	
GA / FRONT PANEL TYPE	/
FRONT PANEL COLOR	
GA / BACK PANEL TYPE	/
BACK PANEL COLOR	
FASCIA TRIM	
FASCIA TRIM COLOR	

06/11/2024

REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH

FOR CONSTRUCTION

DEFLECTION LIMITS:	
EW COLUMN:	80
EW RAFTER LIVE:	180
EW RAFTER WIND:	180
WALL GIRT:	90
PURLIN LIVE:	180
PURLIN WIND:	150
WALL PANEL:	120
ROOF PANEL LIVE:	150
ROOF PANEL WIND:	120
RF HORIZONTAL:	60
RF VERTICAL:	180
WIND BENT:	60
RF CRANE:	100
RF SEISMIC:	65
WIND BENT SEIS.:	65

DELIVERY

1. Customer is responsible for verifying that the Goods listed on the Bill of Lading are received. All shortages and/or damages must be noted, in writing, on the Bill of Lading prior to Buyer signing the Bill of Lading. Failure by the Customer to document shortages of the number of packages or damages within (5) days of delivery or pickup shall waive any claim of such shortage and/or damage. It is Customer's responsibility to retain a copy of the Bill of Lading documenting any shortages and/or damages. Loss of the Bill of Lading shall also waive any right to claim any shortage and/or damage.
2. Building Supplier is not obligated to send Goods by overnight air freight, direct truck line, or other expedited method unless Buyer prepays for such services. Building Supplier shall not be responsible for loss or damage to Goods that occur after tender for pick up or delivery. Seller shall have no obligation to remove or dismantle defective parts or to erect or install replacement parts. Back charges that are not accepted by Building Supplier in writing shall have no effect and Buyer's account may be placed on immediate Credit Hold until resolution. Building Supplier shall not be responsible or financially liable for delivery delays or any of Customer's costs expended on remedies unauthorized by Building Supplier, including, but not limited to, Customer's erection crew expense or rental equipment costs or liquidated or consequential damages of any kind.
3. In the event that parts are damaged during transit, pictures including piece marks should be taken and reported immediately to the Buyer. A replacement part and redelivery date will be coordinated with the manufacturer. Any missing parts should be circled on the Bill of Lading and returned to the driver and reported to the Buyer for immediate resolution

ERECTION NOTES

1. All bracing shown and provided by Building Supplier for this building is required and shall be installed by the ERECTOR as a permanent part of the structure ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 15th edition Manual; Section 7.9).
2. Temporary supports, such as guys, braces, false work, cribbing or other elements required for the erection operation shall be determined and furnished by the ERECTOR ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 15th edition Manual; Section 7.9).
3. Normal erection operations include the correction of minor misfits by moderate amounts of reaming, chipping, or cutting and the drawing of elements into line through use of drift pins. Errors which require major changes in the member configuration are to be reported immediately to Building Supplier by the CUSTOMER to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC 15th edition Manual; Section 7.12).
4. Erection tolerances are set forth in AISC Code of Standard Practice 7.11 except that individual members are considered plumb, level and aligned if the deviation does not exceed 1:300. Variations in finished overall dimensions of structural steel framing are deemed within the limits of good practice when they do not exceed the cumulative effect of rolling, fabricating, and erection tolerances. 4.1. When crane support systems are part of the metal building system erection tolerances Section 9, Common Industry Practices, 2018 MBSM Manual shall apply. To achieve the required tolerances grouting of the columns and shimming of the runway beams may be required. The CUSTOMER shall provide grout if required. The CONTRACTOR erecting the runway beams is responsible for shimming, plumbing, and leveling of the runway system. When aligning the runway beams the alignment shall be with respect to the beam webs so that the center of the aligned rail is over the runway web.
5. As a general rule field welding is not used to assemble a metal building system. In cases where the drawings indicate field welding and in cases where approved corrections are to be made by field welding the following requirements shall be met: 5.1. Welders must be qualified by an independent testing agency, with suitable documentation to AWS D1.1 Structural Welding Code Steel or AWS D1.3 Structural Welding Code – Sheet Steel as applicable, for the processes, positions, and materials involved. 5.2. All welds must be made in conformance to a documented and approved Welding Procedure Specification (WPS). All joints which are not pre-qualified must be supported by a certified Procedure Qualification Record (PQR) by an independent testing agency.
6. All documentation and records shall be the responsibility of the CUSTOMER.
7. Neither Building Supplier nor the CUSTOMER will cut, drill or otherwise alter their work, or the work of other trades to accommodate other trades unless such work is clearly specified in the contract documents. Whenever such work is specified the CUSTOMER is responsible for furnishing complete information as to materials, size, location, and number of alterations prior to preparation of shop drawings ("Code of Standard Practice for Steel Buildings and Bridges" in the AISC Manual; Section 7.13).
8. Field Modifications Policy:
   - 8.1. Building Supplier will only be responsible for the field-modified parts designed and approved by the Building Supplier's Engineering Department.
   - 8.2. Any field modifications designed by third parties may not be approved by Building Supplier and may limit Building Supplier's warranty and liability
   - 8.3. Building Supplier makes no warranty and hereby disclaims any responsibility with respect to the design, engineering, or construction of any field-modified parts performed by third parties.
9. The correction of minor misfits by the use of drift pins to draw the components into line, shimming, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim.
10. Visible gaps between column and/or rafter connection plates can occur as a result of various causes without critical effect to the structural integrity. Minimal shimming at bolt locations is considered acceptable regardless of material yield and does not require full surface contact of the connection plates. The purpose of shimming, besides any aesthetic benefits, is to provide resistance to the tightening procedures of high-strength bolts for proper installation. The types of shim can be of a uniform thickness, full size, tapered or notched around bolts to permit installation without removal of bolts. Bolt holes oversized by 1/16 inches are permitted in full-size shims to facilitate alignment. For further information regarding shimming, refer to the AISC publication, "Engineering for Steel Construction". In the event of connection gaps, the manufacturer must be consulted for approval and specific recommendations for proper shimming.
11. The Building Supplier, through its CS Manager, must be notified at once when a condition becomes apparent that may result in a backcharge by the Erector. Notification by phone must be confirmed in writing. Some approximation of the amount of the backcharge must be established at this time and an authorization before the work is started. Building Supplier will not honor any field corrections or backcharges unless prior notice has been given and agreed upon. All discrepancies must be agreed upon, in writing, Any work which is undertaken without such notification and authorization will not be honored as a backcharge.
    - 11.1 Description of nature and extent of the errors, including piece marks, quantities, photos, and measurements, where applicable.
    - 11.2 Description of nature and extent of proposed corrective work including estimated man-hours.
    - 11.3 Material to be purchased from other than the Building Supplier, including estimated quantities and cost.
12. WARNING in no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run-off from copper flashing, wiring, or tubing onto Galvalume should be avoided.
13. It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State and Federal safety and health standards should always be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.
14. Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.
15. Roof snow accumulations in excess of specified project design loading criteria can cause significant distress to the building structural system. It is recommended that roof be cleared of snow by the CUSTOMER Refer to A9.4 for Snow/Ice Removal procedure by Metal Building Systems Manual. A copy is available upon request.

DRAWING INDEX

BUILDING DESCRIPTION	
WIDTH (FT)	80
LENGTH (FT)	80
BACK SIDE WALL EAVE HEIGHT (FT)	23
FRONT SIDE WALL EAVE HEIGHT (FT)	23
BACK SIDE WALL ROOF SLOPE	1.0:12
FRONT SIDE WALL ROOF SLOPE	1.0:12
BAY SPACING (FT)	SEE PLAN

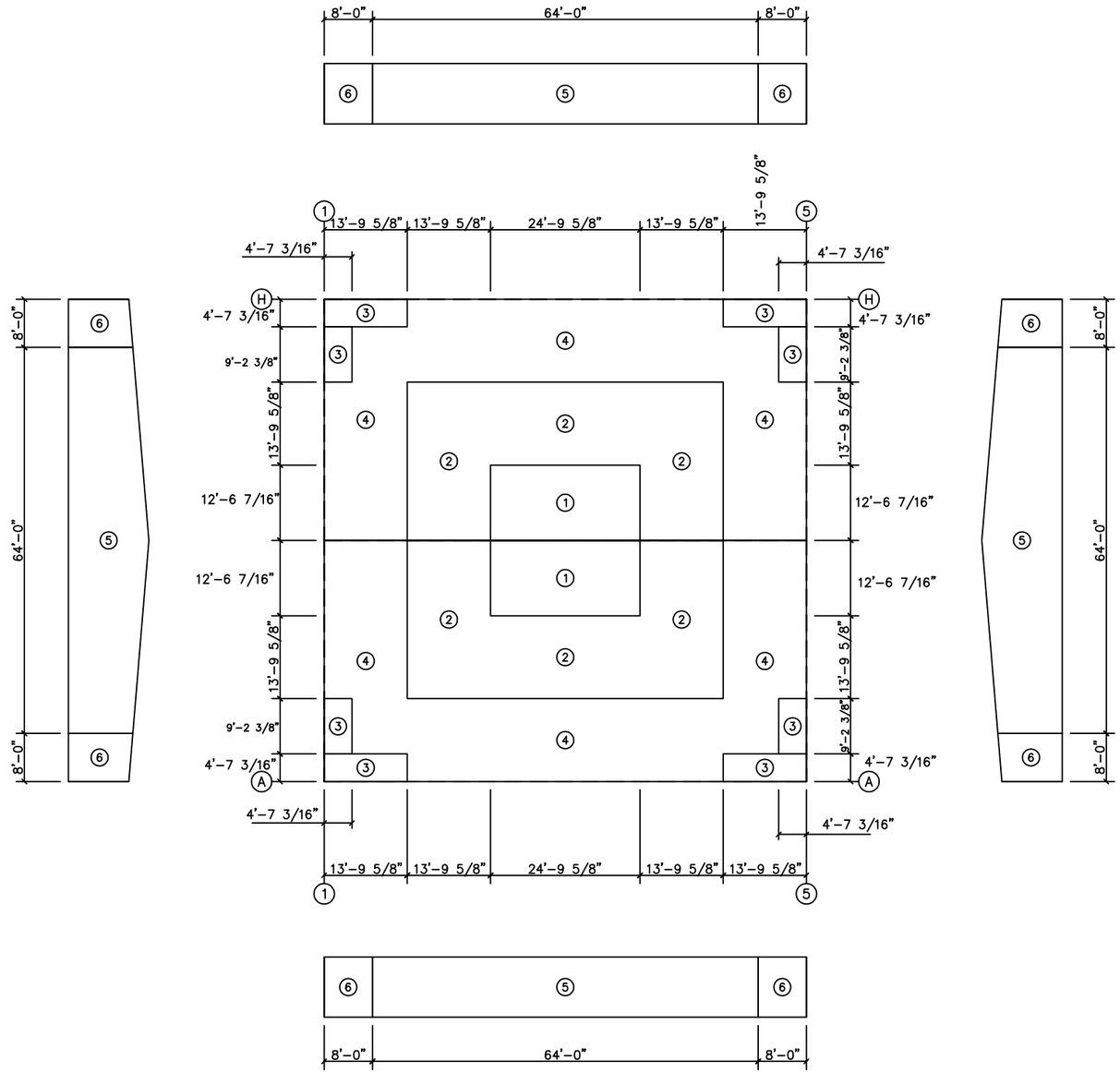
ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	5/8/2024	RAK	VBA	GFA
0	FOR CONSTRUCTION	5/10/2024	RAK	VBA	GFA

METAL BUILDING OUTLET CORP.

BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	C1

COMPONENTS AND CLADDING LOAD GROUP: Standard				
	Member		Panel	
O Id	Pressure	Suction	Pressure	Suction
1	9.60	−13.25	9.60	−15.40
2	9.60	−19.22	9.60	−26.80
3	9.60	−28.49	9.60	−48.21
4	9.60	−25.58	9.60	−35.42
5	12.46	−13.73	15.40	−16.69
6	12.46	−14.69	15.40	−20.53



PANEL ZONE LAYOUT
(Wind Pressures, Factored (psf))

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
0	FOR CONSTRUCTION	05/08/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

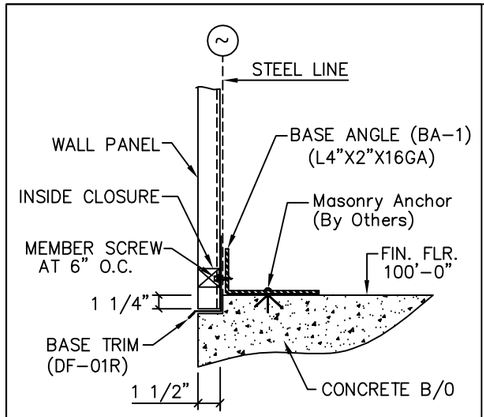
DESCRIPTION	COMPONENTS AND CLADDING WIND ZONE
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	C2 OF C2

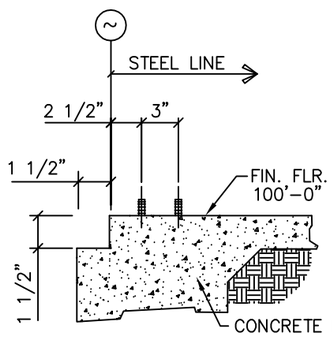
FOR CONSTRUCTION

06/11/2024

REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH

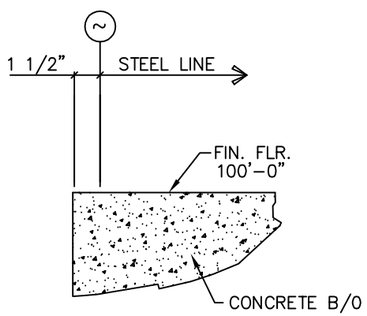


SECTION "A" NTS



FIELD LOCATE
WALK DOOR

SECTION "B" NTS



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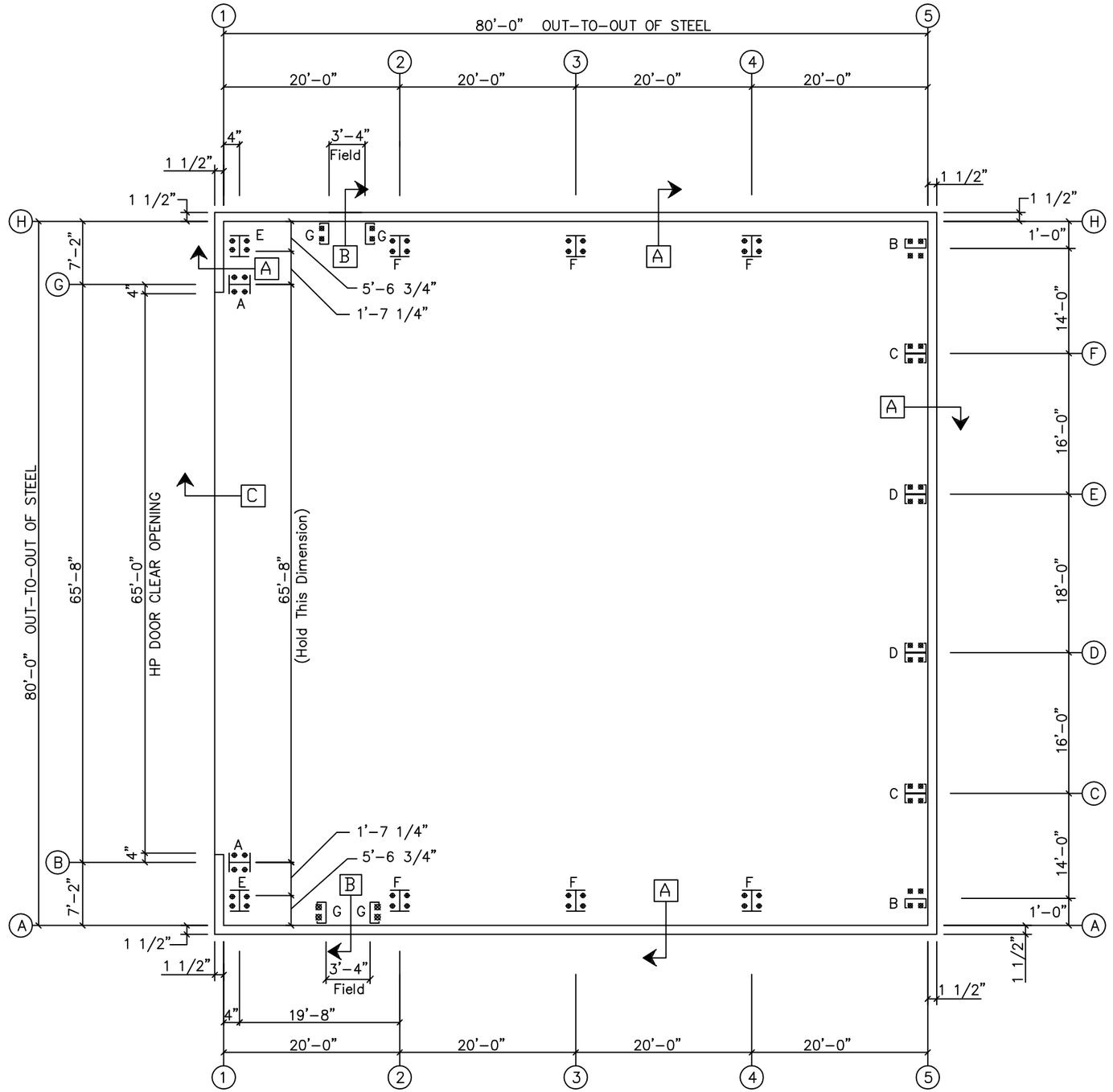


Dia= 5/8"

Dia= 3/4"

ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
0	FOR CONSTRUCTION	05/08/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

DESCRIPTION	ANCHOR BOLT PLAN
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

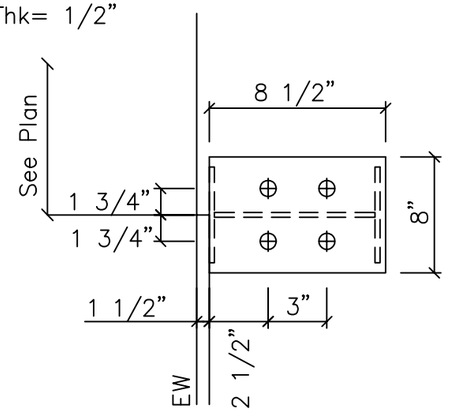
S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	F1 OF F3

FOR CONSTRUCTION

06/11/2024

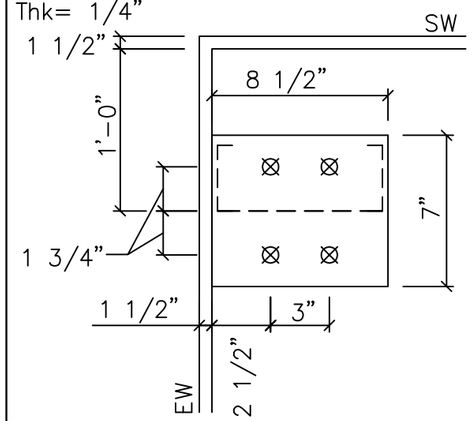
REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH

Dia= 3/4"
Thk= 1/2"



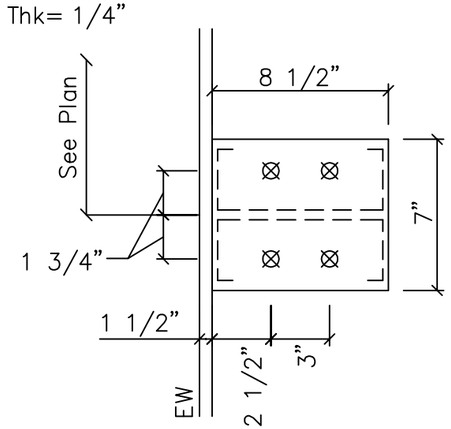
DETAIL A

Dia= 5/8"
Thk= 1/4"



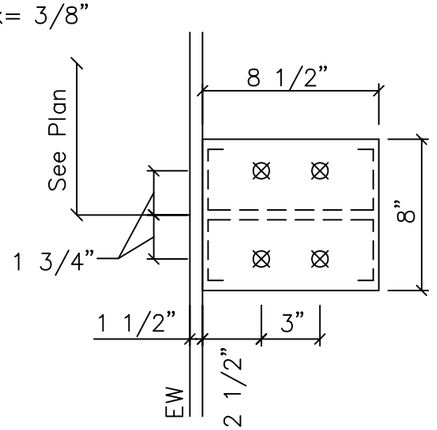
DETAIL B

Dia= 5/8"
Thk= 1/4"



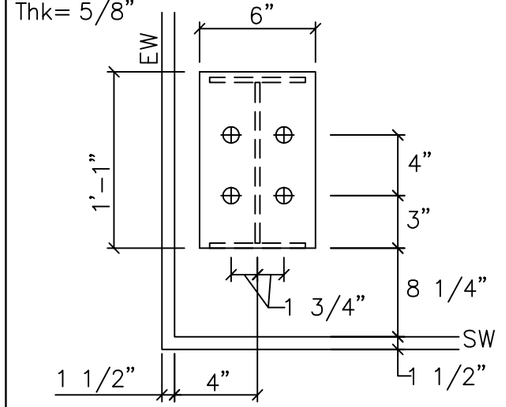
DETAIL C

Dia= 5/8"
Thk= 3/8"



DETAIL D

Dia= 3/4"
Thk= 5/8"



DETAIL E

Dia= 3/4"
Thk= 1/2"

DETAIL F

Dia= 5/8"
Thk= 3/8"

DETAIL G

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
0	FOR CONSTRUCTION	05/08/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

DESCRIPTION	COMPONENTS AND CLADDING WIND ZONE
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	F2 OF F3

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FOR CONSTRUCTION

06/11/2024

REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width	(ft)	=	80.0
Length	(ft)	=	80.0
Eave Height	(ft)	=	23.0/ 23.0
Roof Slope	(rise/12)	=	1.00/ 1.00
Roof Dead Load	(psf)	=	2.0
Wall Dead Load			
Left Endwall	(psf)	=	2.0
Right Endwall	(psf)	=	2.0
Front Sidewall	(psf)	=	2.0
Back Sidewall	(psf)	=	2.0
Live Load	(psf)	=	20.0
Collateral Load	(psf)	=	3.0
Snow Load	(psf)	=	28.0
Wind Speed	(mph)	=	115.0
Wind Code		=	IBC 21
Exposure		=	C
Closure		=	Enclosed
Internal Wind Coeff		=	−0.18, +0.18
Risk Category		=	II − Normal
Importance − Wind		=	1.00
Importance − Seismic		=	1.00
Seismic Design Category		=	D
Seismic Coeff	(Sms)	=	0.77

ID	Description
1	Dead+Collateral+Snow+Slide_Snow
2	Dead+0.6Wind_Left1
3	Dead+0.6Wind_Right1
4	0.6Dead+0.6Wind_Left1
5	0.6Dead+0.6Wind_Right1
6	0.6Dead+0.6Wind_Left2
7	0.6Dead+0.6Wind_Right2
8	0.6Dead+0.6Wind_Long1L
9	0.6Dead+0.6Wind_Long2L
10	Dead+Collateral+F1UNB_SL_L
11	Dead+Collateral+F1UNB_SL_R
12	Dead+Collateral
13	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
14	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
15	Dead+Collateral+E1UNB_SL_L
16	0.6Dead+0.6Wind_Right1+0.6Wind_Suction
17	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
18	Dead+Collateral+E1UNB_SL_R
19	0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
20	Dead+Collateral+E2UNB_SL_L
21	Dead+Collateral+E2UNB_SL_R
22	0.6Dead+0.6Wind_Suction+0.6Wind_Long2L

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
⊠ 8	Jamb	5/8"	A307	2.00
⊕ 8	Endwall	3/4"	A307	2.50
⊠ 24	Endwall	5/8"	A307	2.00
⊕ 32	Frame	3/4"	A307	2.50

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	1								(h)
F_SW	A	2,3	8.9	9.5	5.4	5.7			
R_EW	5	D,E	2.4	3.2	1.7	2.2			
B_SW	H	3,2	8.9	9.5	5.4	5.7			

(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Snow Horz	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert
1	H	0.0	−2.6	0.0	−1.7	0.1	−11.3	0.1	−15.8	−2.6	8.8	2.7	16.3
1	A	0.0	−2.6	0.0	−1.7	−0.1	−11.3	−0.1	−15.8	−2.7	16.3	2.6	8.8
1	@ 7.2	0.0	5.3	0.0	2.9	0.0	19.5	0.0	27.2	0.0	−19.9	0.0	−23.9
1	@72.8	0.0	5.3	0.0	2.9	0.0	19.5	0.0	27.2	0.0	−23.9	0.0	−19.9

Frame Line	Column Line	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert
1	H	−3.2	5.2	2.0	12.7	1.3	8.4	1.0	6.2	−0.8	−3.0	0.8	3.0
1	A	−2.0	12.7	3.2	5.2	−1.0	6.2	−1.3	8.4	−0.8	3.0	0.8	−3.0
1	@ 7.2	0.0	−12.8	0.0	−16.8	0.0	−16.4	0.0	−12.1	0.0	3.1	0.0	−3.1
1	@72.8	0.0	−16.8	0.0	−12.8	0.0	−12.1	0.0	−16.4	0.0	−3.1	0.0	3.1

Frame Line	Column Line	MIN_SNOW Horz	MIN_SNOW Vert	F1UNB_SL_L Horz	F1UNB_SL_L Vert	F1UNB_SL_R Horz	F1UNB_SL_R Vert
1	H	0.1	−11.3	0.1	−16.4	0.1	−11.2
1	A	−0.1	−11.3	−0.1	−11.2	−0.1	−16.4
1	@ 7.2	0.0	19.5	0.0	28.8	0.0	16.8
1	@72.8	0.0	19.5	0.0	16.8	0.0	28.8

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Snow Horz	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert
2*	H	1.5	3.2	1.4	2.4	9.5	16.0	13.3	22.4	−11.2	−16.1	−3.3	−10.8
2*	A	−1.5	3.2	−1.4	2.4	−9.5	16.0	−13.3	22.4	3.3	−10.8	11.2	−16.1

Frame Line	Column Line	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert
2*	H	−9.3	−9.3	−1.4	−4.0	−4.3	−24.4	−4.8	−21.6	−1.2	−0.7	1.2	0.7
2*	A	1.4	−4.0	9.3	−9.3	4.8	−21.6	4.3	−24.4	−1.2	0.7	1.2	−0.7

Frame Line	Column Line	Seismic_Long Horz	Seismic_Long Vert	MIN_SNOW Horz	MIN_SNOW Vert	F2UNB_SL_L Horz	F2UNB_SL_L Vert	F2UNB_SL_R Horz	F2UNB_SL_R Vert
2*	H	0.0	−5.7	9.5	16.0	11.1	22.4	11.1	12.7
2*	A	0.0	−5.7	−9.5	16.0	−11.1	12.7	−11.1	22.4

2* Frame lines: 2 3 4

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert
1	G	2.1	2.8	18.6	26.0	−24.1	−18.8	−16.5	−11.1	−7.9	8.8	−23.4	−18.0
1	B	2.1	2.8	18.6	26.0	−18.8	−24.1	−11.1	−16.5	−7.9	8.8	−18.0	−23.4

Frm Line	Col Line	Seis Left Vert	Seis Right Vert	Seis Long Vert	MIN_SNOW Horz	MIN_SNOW Vert	E1UNB_SL_L Horz	E1UNB_SL_L Vert	E1UNB_SL_R Horz	E1UNB_SL_R Vert
1	G	−0.2	−0.3	0.0	0.0	18.5	0.0	27.3	0.0	15.9
1	B	−0.3	−0.2	0.0	0.0	18.5	0.0	15.9	0.0	27.3

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
5	A	0.3	0.2	1.3	1.8	0.0	−1.8	0.0	−1.2	0.0	−1.1	0.0	−0.6	−1.8
5	C	0.7	0.5	3.4	4.7	0.0	−5.0	0.0	−2.8	0.0	−3.5	0.0	−1.3	−3.6
5	D	0.7	0.5	3.5	4.9	−2.4	−8.2	0.0	0.3	−2.4	−6.8	0.0	1.6	−4.2
5	E	0.7	0.5	3.5	4.9	0.0	0.3	2.4	−8.2	0.0	1.6	2.4	−6.8	−4.2
5	F	0.7	0.5	3.4	4.7	0.0	−2.8	0.0	−5.0	0.0	−1.3	0.0	−3.5	−3.6
5	H	0.3	0.2	1.3	1.8	0.0	−1.2	0.0	−1.8	0.0	−0.6	0.0	−1.1	−1.8

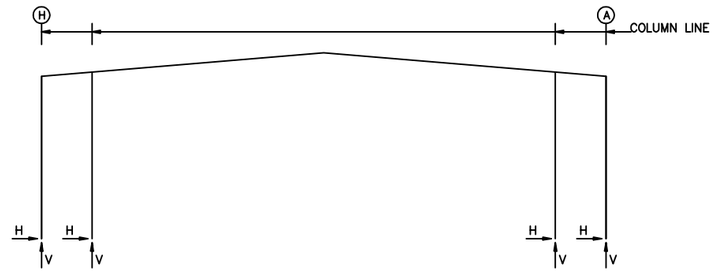
Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	Seis Long Horz	MIN_SNOW Horz	MIN_SNOW Vert
5	A	2.1	0.0	−2.0	0.0	−1.2	0.0	0.0	0.0	0.0	0.0	0.0	1.3
5	C	3.9	0.0	−5.1	0.0	−2.8	0.0	0.1	0.0	0.0	0.1	0.0	3.4
5	D	4.7	0.0	−4.1	−0.4	−3.5	−1.7	−2.3	0.0	2.3	0.1	0.0	3.5
5	E	4.7	0.4	−3.5	0.0	−4.1	0.0	2.3	1.7	−2.3	0.1	0.0	3.5
5	F	3.9	0.0	−2.8	0.0	−5.1	0.0	0.0	0.0	0.1	0.1	0.0	3.4
5	H	2.1	0.0	−1.2	0.0	−2.0	0.0	0.0	0.0	0.0	0.0	0.0	1.3

Frm Line	Col Line	E2UNB_SL_L Horz	E2UNB_SL_L Vert	E2UNB_SL_R Horz	E2UNB_SL_R Vert
5	A	0.0	1.7	0.0	0.6
5	C	0.0	5.5	0.0	1.3
5	D	0.0	6.8	0.0	2.0
5	E	0.0	2.0	0.0	6.8
5	F	0.0	1.3	0.0	5.5
5	H	0.0	0.6	0.0	1.7

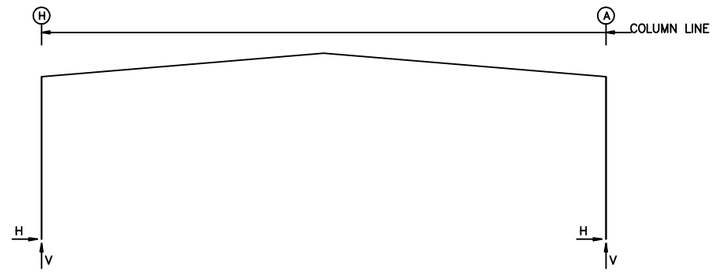
ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Grout (in)
1	G	13	5.3	−12.7	14	−4.8	−12.3						
		15	0.0	31.5	13	5.3	−12.7						
1	B	16	5.3	−12.7	17	−4.8	−12.3						
		18	0.0	31.5	16	5.3	−12.7						
5	A	19	1.2	−1.0	14	−1.1	−1.0	4	0.625	7.000	8.500	0.250	0.0
		1	0.0	2.2	19	1.2	−1.0						
5	C	19	2.4	−2.6	14	−2.1	−2.6	4	0.625	7.000	8.500	0.250	0.0
		20	0.0	6.7	19	2.4	−2.6						
5	D	13	2.8	−4.5	14	−2.5	−2.0	4	0.625	8.000	8.500	0.375	0.0
		20	0.0	8.0	13	2.8	−4.5						
5	E	16	2.8	−4.5	17	−2.5	−2.0	4	0.625	8.000	8.500	0.375	0.0
		21	0.0	8.0	16	2.8	−4.5						
5	F	22	2.4	−2.6	17	−2.1	−2.6	4	0.625	7.000	8.500	0.250	0.0
		21	0.0	6.7	22	2.4	−2.6						
5	H	22	1.2	−1.0	17	−1.1	−1.0	4	0.625	7.000	8.500	0.250	0.0
		1	0.0	2.2	22	1.2	−1.0						

FRAME LINES: 1



FRAME LINES: 2 3 4



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Grout (in)
1	H	3	1.6	7.2	6	−1.9	1.6	4	0.750	6.000	13.00	0.625	0.0
		5	1.6	8.2	10	0.2	−20.7						
1	A	7	1.9	1.6	2	−1.6	7.2	4	0.750	6.000	13.00	0.625	0.0
		4	−1.6	8.2	11	−0.2	−20.7						
1	G	5	0.0	−11.2	5	0.0	−11.2	4	0.750	8.000	9.00	0.500	0.0
		12	0.0	37.0									
1	B	4	0.0	−11.2	4	0.0	−11.2	4	0.750	8.000	9.00	0.500	0.0
		12	0.0	37.0									

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick	Grout (in)
2*	H	1	16.3	28.0	4	−5.8	−7.8	4	0.750	6.000	11.00	0.500	0.0
					8	−1.7	−12.7						
2*	A	5	5.8	−7.8	1	−16.3	28.0	4	0.750	6.000	11.00	0.500	0.0
		1	−16.3	28.0	9	1.7	−12.7						

2* Frame lines: 2 3 4

06/11/2024

REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH

FOR CONSTRUCTION

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
0	FOR CONSTRUCTION	05/08/2024	VBA	RAK	GFA

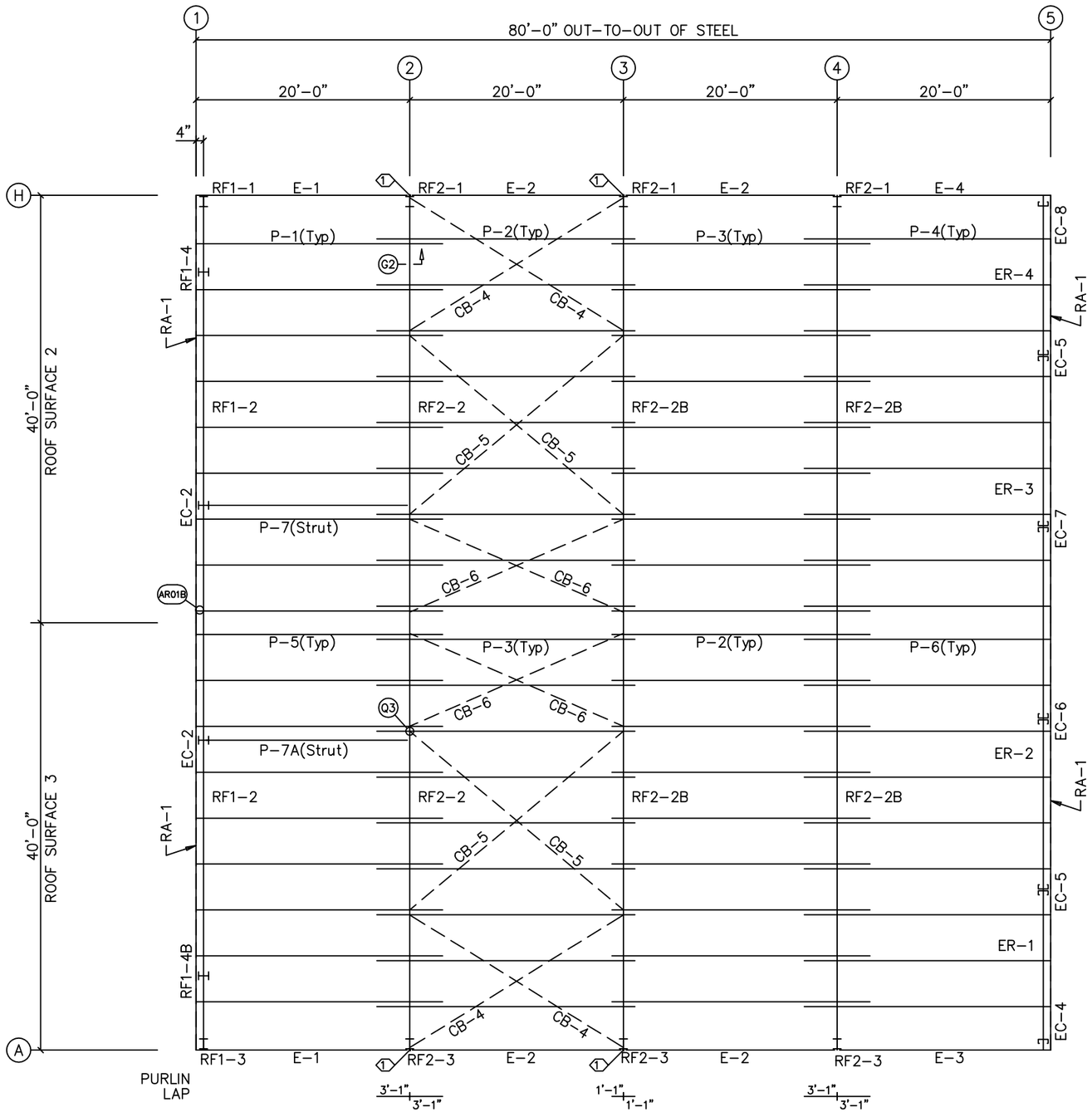
METAL BUILDING OUTLET CORP.

DESCRIPTION	ANCHOR BOLT REACTION
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.# 112949 JOB# 112949 SCALE N.T.S. DWG# F3 OF F3

SPECIAL BOLTS ROOF PLAN					
ID	QUAN	TYPE	DIA	LENGTH	WASH
1	4	A325	1/2"	1 1/4"	0

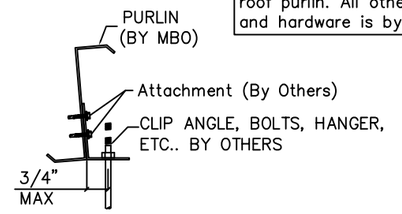
MEMBER TABLE ROOF PLAN		
QUAN	MARK	PART
9	P-1	8x25Z14
18	P-2	8x25Z16
18	P-3	8x25Z16
9	P-4	8x25Z14
9	P-5	8x25Z14
9	P-6	8x25Z14
1	P-7	8X35C12
1	P-7A	8X35C12
2	E-1	E0852.7541L
4	E-2	E0852.7541L
1	E-3	E0852.7541L
1	E-4	E0852.7541L
4	CB-4	BR5/8
4	CB-5	BR1/2
4	CB-6	BR1/2



ROOF FRAMING PLAN

6" ROOF INSULATION (BY OTHERS)

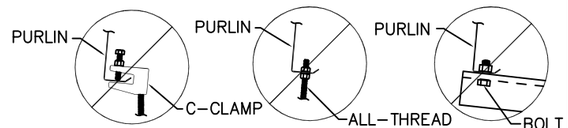
NOTE: MBO only provides the roof purlin. All other material and hardware is by others.



Recommended Connection Detail

NOTE:
Drilling of holes in flanges of purlins is structurally detrimental to the member. Any collateral loads, which are included in the purlin design loads, are to be attachedby connection directly to the web only.

Field Modifications – Warning



Flange C-Clamp is not an acceptable connection

Connection through the flange is not acceptable

Connection through the flange is not acceptable

ATTACHMENT OF SUSPENDED ITEMS (BY OTHERS)

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

DESCRIPTION	ROOF FRAMING PLAN						
BUYER / CUSTOMER	RUSSEL KEY 52F						
END USER	RUSSEL KEY 52F						
END USE	Aviation						
STREET	4550 S AIRPORT LOT 52F						
CITY, STATE, ZIP	ST GEORGE UT 84790						
COUNTY	WASHINGTON						
S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	E1 OF E9

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06/11/2024

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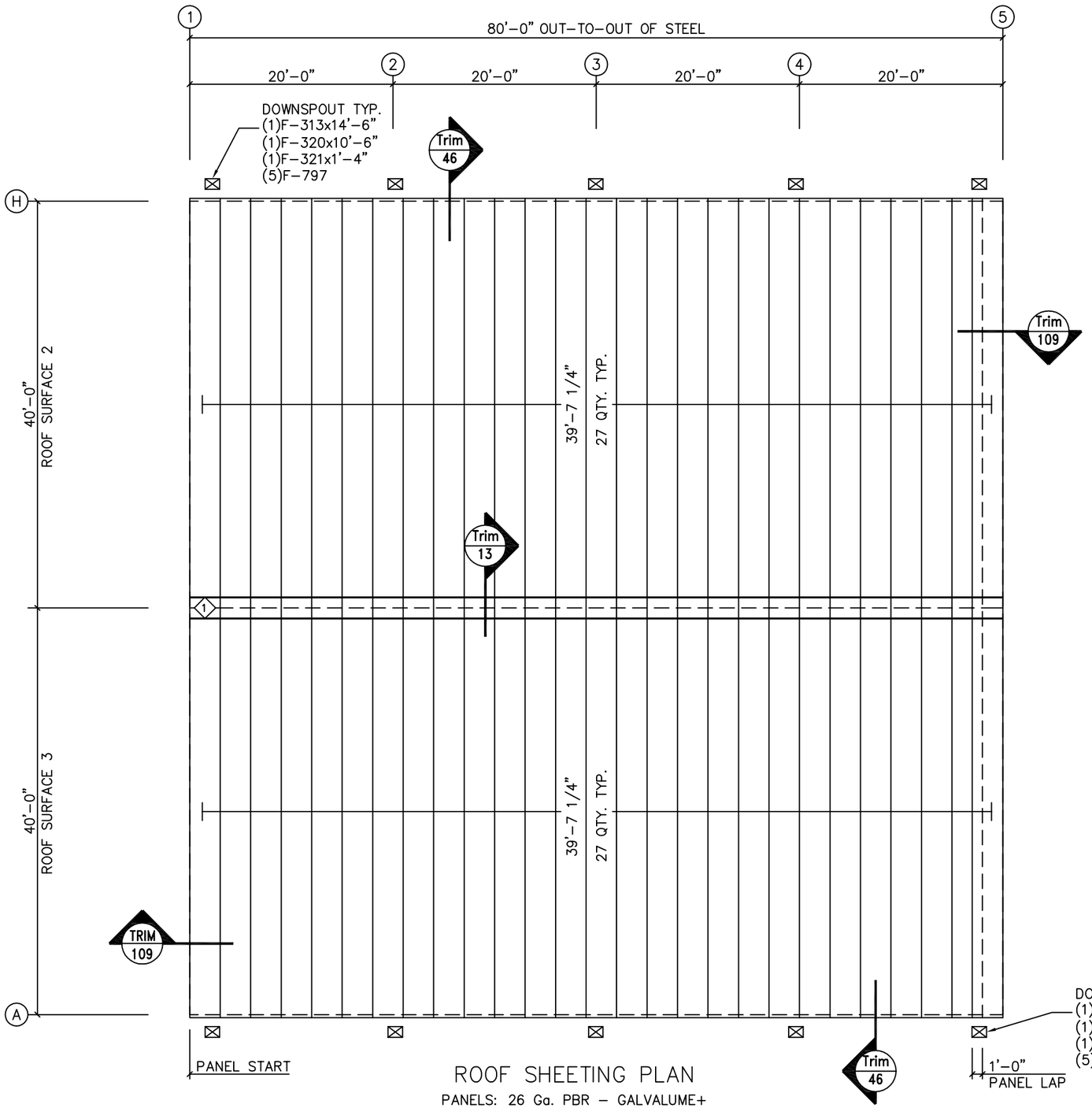
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH

TRIM TABLE ROOF PLAN					
◇ID	QUAN	PART	LENGTH	COLOR	DETAIL
1	27	RPR–02	3'–0"	GALVALUME+	TRIM_13



ROOF SHEETING PLAN

PANELS: 26 Ga. PBR – GALVALUME+

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

DESCRIPTION	ROOF SHEETING PLAN
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	E2 OF E9

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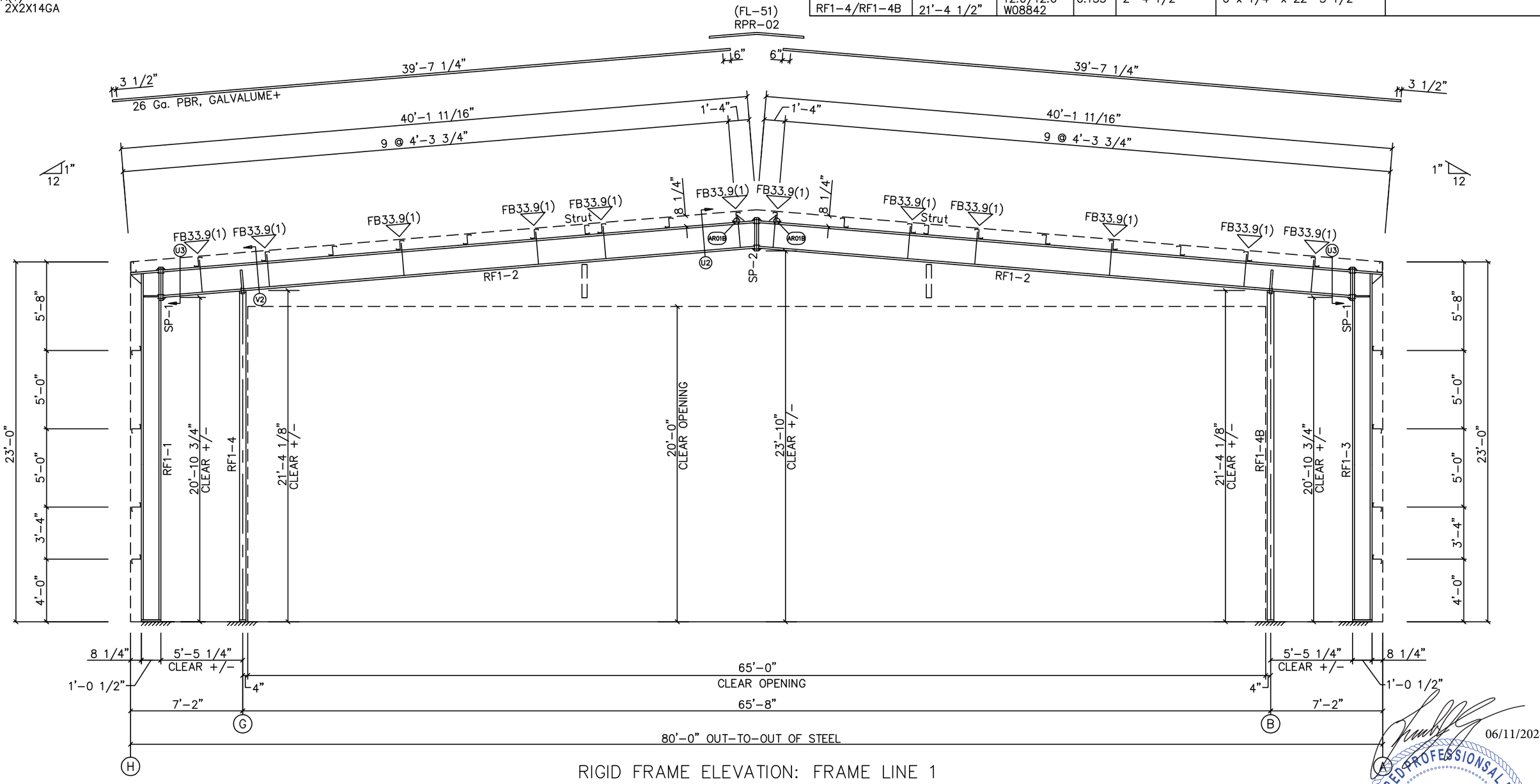
06/11/2024

REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH

SPLICE PLATE & BOLT TABLE										CAP PLATE BOLTS				
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length	Mark	Qty	Type	Dia	Length
SP-1	4	4	0	A325	0.625	2.00	6"	1/2"	2'-0 15/16"	RF1-4	4	A325	0.625	1.75
SP-2	4	4	0	A325	0.625	2.00	6"	1/2"	2'-0 13/16"					

FLANGE BRACES: Both Sides(U.N.)
FBxxA(1)
A – 2X2X14GA

MEMBER TABLE							
Mark	Length	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	22'-4 1/2"	12.0/12.0	0.135	2'-4 1/2"	6 x 1/4" x 22'-3 1/2"	6 x 1/4" x 20'-7 1/16"
		12.0/12.0	0.135	20'-0"	6 x 5/16" x 1'-8 9/16"	
RF1-2	38'-4 9/16"	18.0/18.0	0.188	18'-5 1/16"	6 x 5/16" x 6'-3 1/4"	6 x 3/8" x 6'-4 3/4"
		18.0/18.0	0.135	20'-0"	6 x 3/8" x 12'-0 5/16"	6 x 1/2" x 12'-0 5/16"
					6 x 1/4" x 20'-0"	6 x 1/4" x 19'-10 1/2"
RF1-3	22'-4 1/2"	12.0/12.0	0.135	20'-0"	6 x 5/16" x 1'-8 9/16"	6 x 1/4" x 20'-7 1/16"
		12.0/12.0	0.135	2'-4 1/2"	6 x 1/4" x 22'-3 1/2"	
RF1-4/RF1-4B	21'-4 1/2"	W08842				



RIGID FRAME ELEVATION: FRAME LINE 1

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

DESCRIPTION	RIGID FRAME ELEVATION
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON
S.O.# 112949	JOB# 112949 SCALE N.T.S. DWG# E3 OF E9

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT THE ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY THE FABRICATOR IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL AND MECHANICAL SYSTEMS AND / OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN THE FABRICATOR ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

FOR CONSTRUCTION

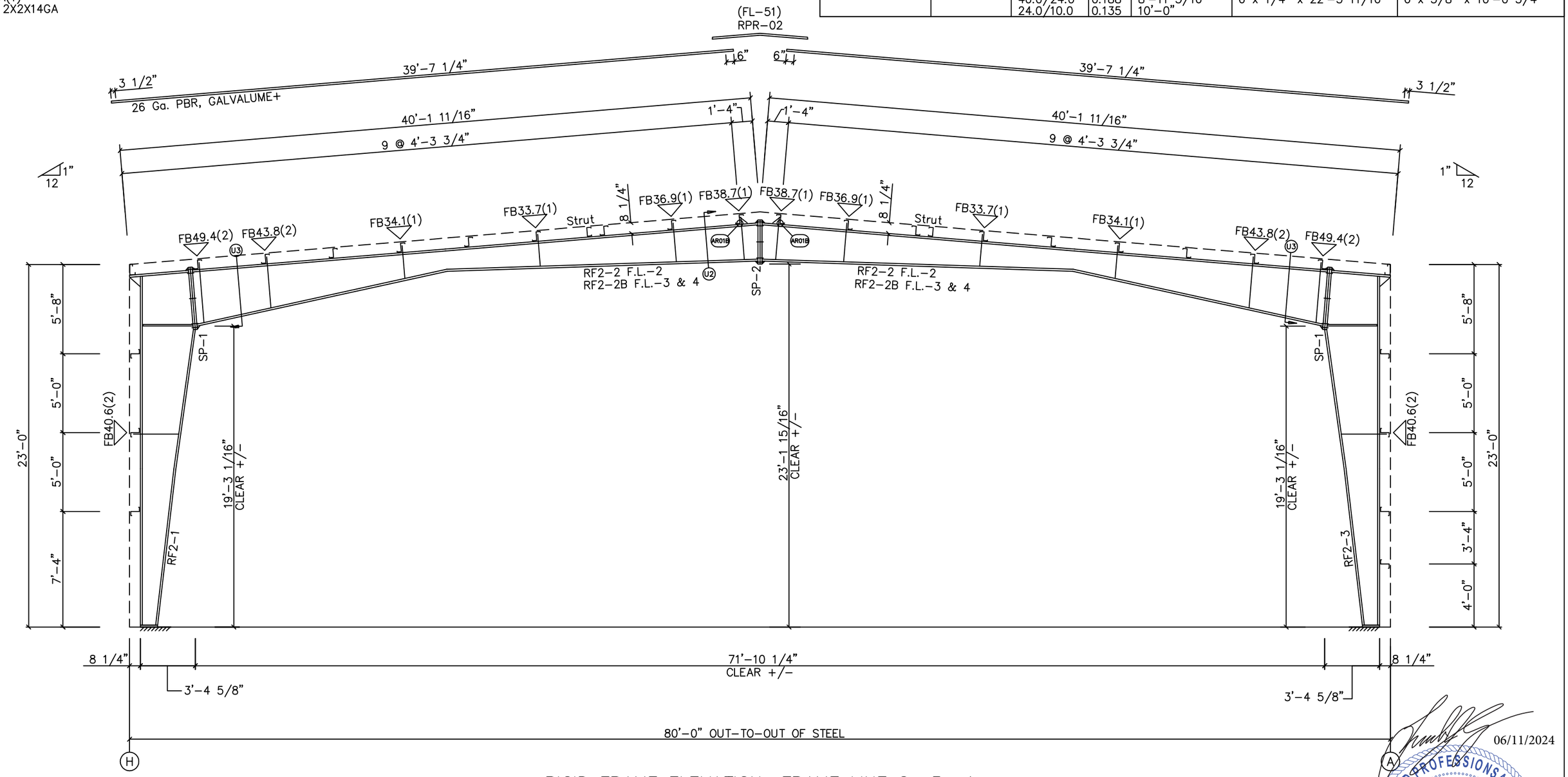
06/11/2024

REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	0.750	2.25	6"	5/8"	3'-10 7/8"
SP-2	4	4	2	A325	0.625	2.00	6"	1/2"	2'-8 15/16"

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1)
A - 2X2X14GA

MEMBER TABLE						
Mark	Length	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF2-1	22'-4 7/16"	10.0/24.0	0.135	10'-0"	6 x 1/4" x 22'-3 11/16"	6 x 3/8" x 10'-0 3/4"
		24.0/40.0	0.188	8'-11 5/16"	6 x 1/4" x 3'-9"	6 x 3/8" x 9'-0 1/2"
		40.0/36.4	0.250	3'-7 3/8"		
RF2-2/RF2-2B	36'-4"	40.0/14.0	0.188	16'-2 7/8"	6 x 1/4" x 16'-2 7/8"	6 x 3/8" x 16'-4 9/16"
		14.0/26.0	0.135	20'-0"	6 x 3/8" x 20'-0"	6 x 5/16" x 19'-10 1/8"
RF2-3	22'-4 7/16"	36.4/40.0	0.250	3'-7 3/8"	6 x 1/4" x 3'-9"	6 x 3/8" x 9'-0 1/2"
		40.0/24.0	0.188	8'-11 5/16"	6 x 1/4" x 22'-3 11/16"	6 x 3/8" x 10'-0 3/4"
		24.0/10.0	0.135	10'-0"		



RIGID FRAME ELEVATION: FRAME LINE 2 3 4

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

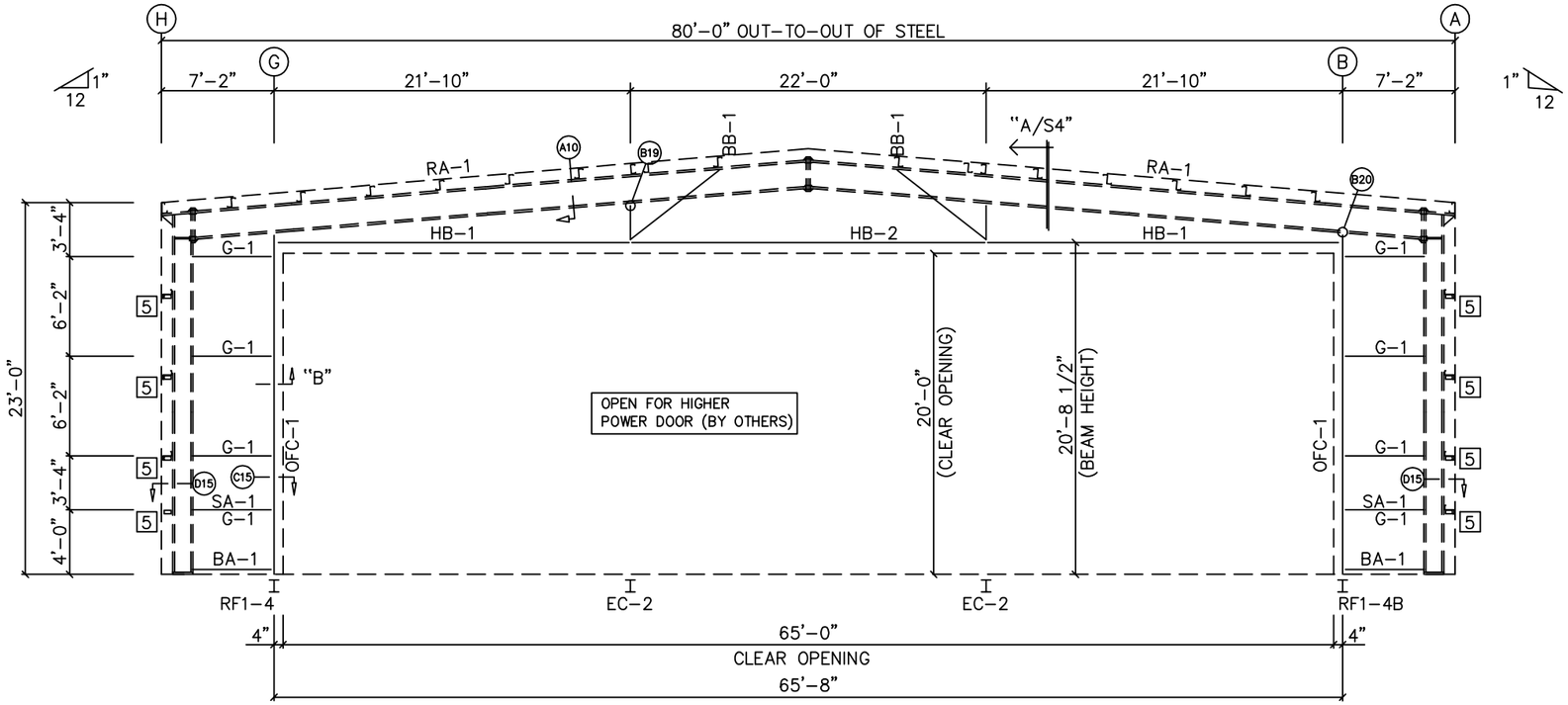
DESCRIPTION	RIGID FRAME ELEVATION
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.# 112949 JOB# 112949 SCALE N.T.S. DWG# E4 OF E9

FOR CONSTRUCTION

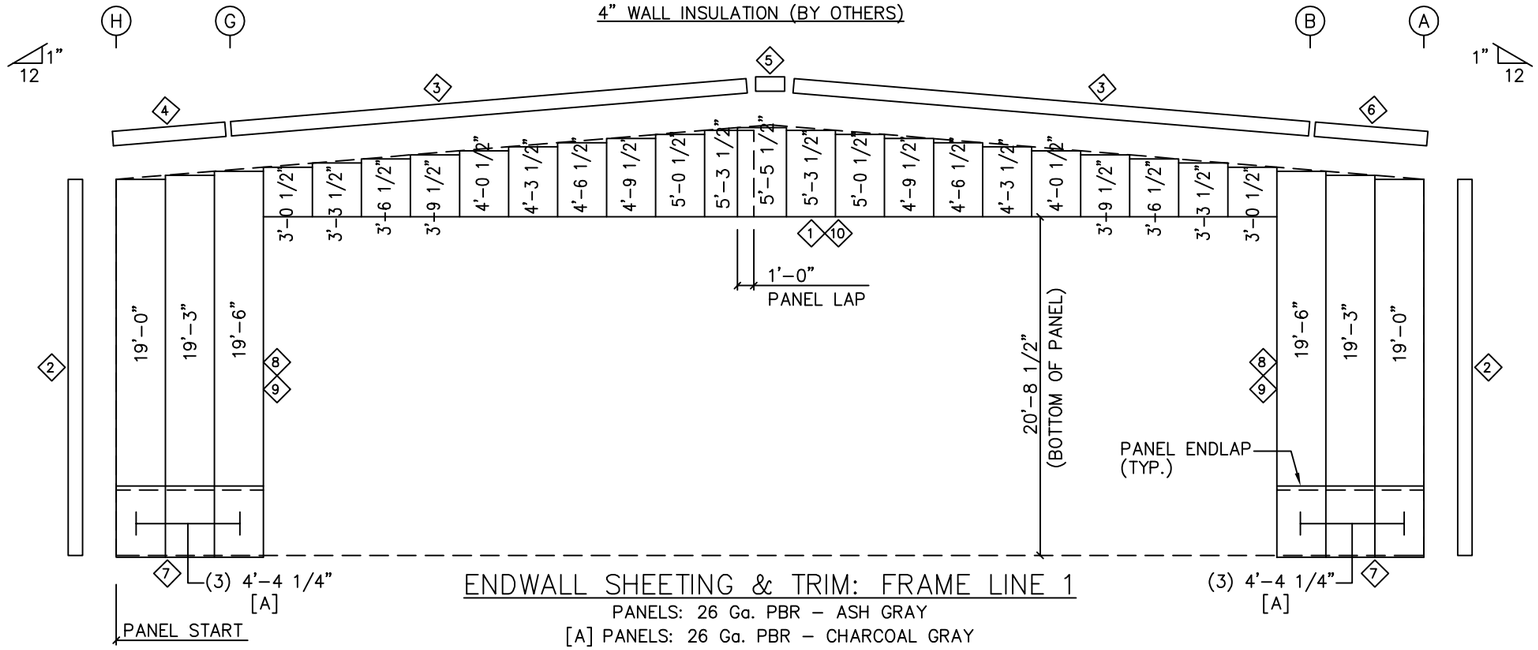
06/11/2024

REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH



ENDWALL FRAMING: FRAME LINE 1

4" WALL INSULATION (BY OTHERS)



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. PBR – ASH GRAY

[A] PANELS: 26 Ga. PBR – CHARCOAL GRAY

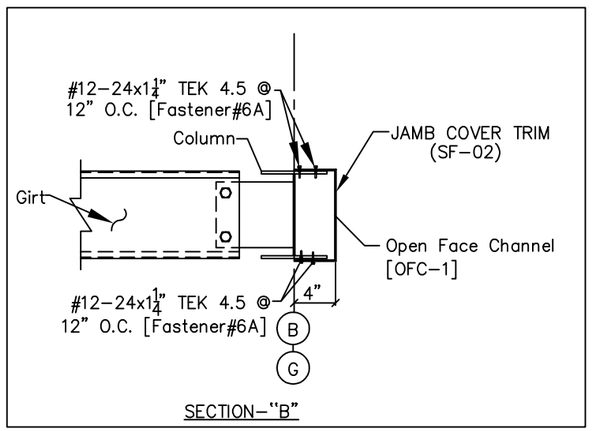
(FIELD CUT WALL PANEL AS REQUIRED)

TRIM TABLE FRAME LINE 1					
◇ID	QUAN	PART	LENGTH	COLOR	DETAIL
1	5	DF-01S	13'-10"	CHARCOAL GRAY	TRIM_3X
2	4	CT-01R	12'-0"	ASH GRAY	TRIM_19
3	4	RT-01	15'-7"	CHARCOAL GRAY	TRIM_109
4	1	RT-01L	11'-1"	CHARCOAL GRAY	TRIM_35
5	1	PB-50	1'-6"	CHARCOAL GRAY	TRIM_12
6	1	RT-01R	11'-1"	CHARCOAL GRAY	TRIM_109
7	2	DF-01R	10'-6"	CHARCOAL GRAY	TRIM_176
8	4	JT-01R	11'-8"	CHARCOAL GRAY	TRIM_2X
9	4	SF-02	11'-8"	CHARCOAL GRAY	TRIM_2X
10	5	SF-01	13'-10"	CHARCOAL GRAY	TRIM_1X

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	4	A325	1/2"	1 1/4"
Back Braces	1	A325	5/8"	1 1/2"

CONNECTION PLATES FRAME LINE 1		
□ID	QUAN	MARK/PART
5	8	CC5

MEMBER TABLE FRAME LINE 1	
MARK	PART
HB-1	8x35C16
HB-2	8x35C16
BB-1	L3x3x3/16
RF1-4	W08842
EC-2	W08542
RF1-4B	W08842
G-1	8x25Z16
OFC-1	8 5/8x3 5/8C14



SECTION-"B"

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

DESCRIPTION	ENDWALL FRAMING & SHEETING
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

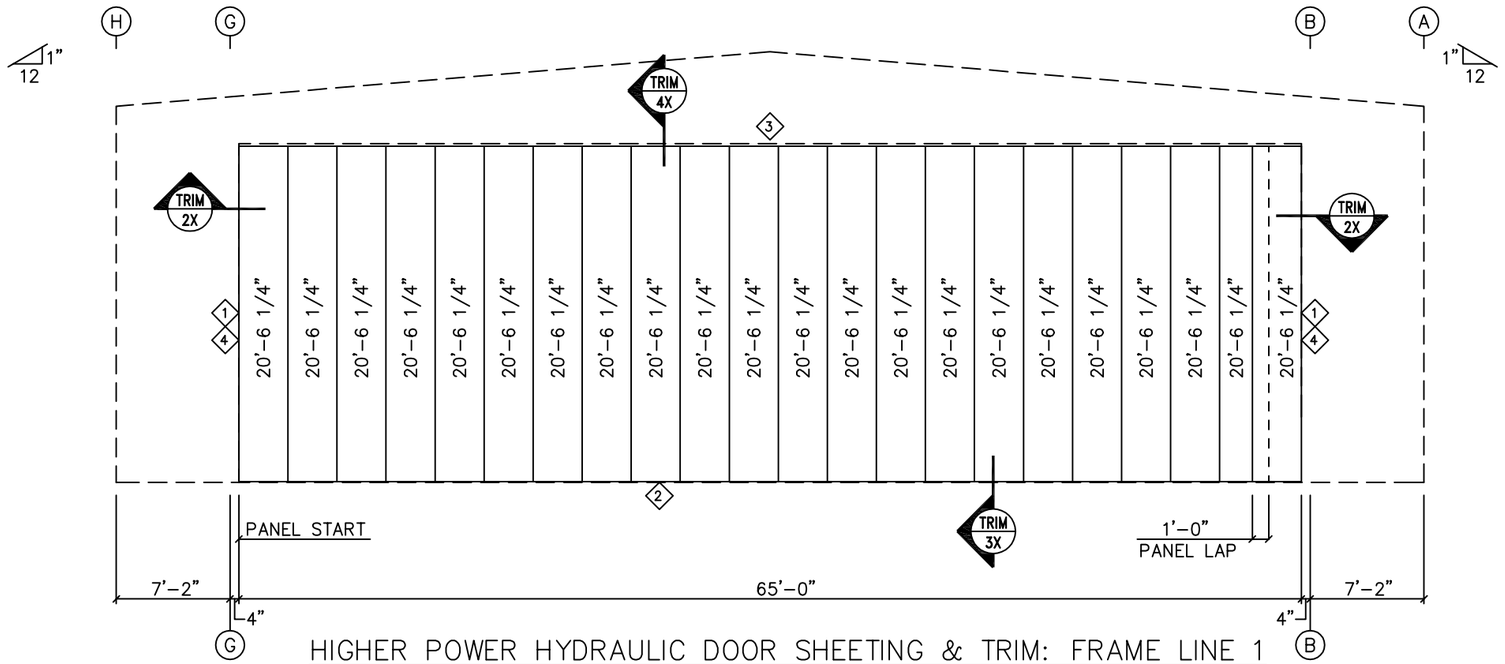
S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	E5 OF E9

FOR CONSTRUCTION

06/11/2024

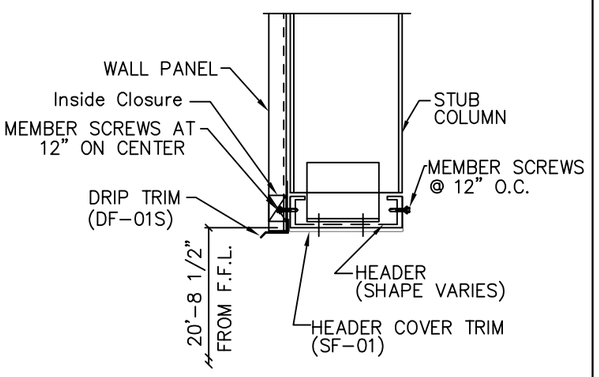
REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH

TRIM TABLE FRAME LINE 1					
◇ID	QUAN	PART	LENGTH	LENGTH	DETAIL
1	4	JT-01R	11'-8"	CHARCOAL GRAY	TRIM_2X
2	5	DF-01S	13'-10"	CHARCOAL GRAY	TRIM_3X
3	5	SF-03	13'-10"	CHARCOAL GRAY	TRIM_4X
4	4	SF-04	11'-8"	CHARCOAL GRAY	TRIM_2X

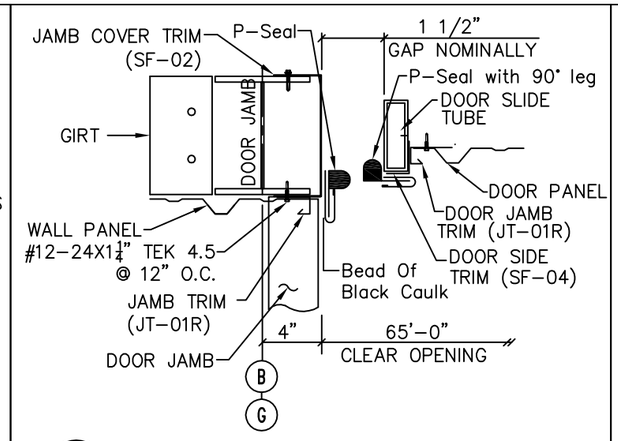


HIGHER POWER HYDRAULIC DOOR SHEETING & TRIM: FRAME LINE 1

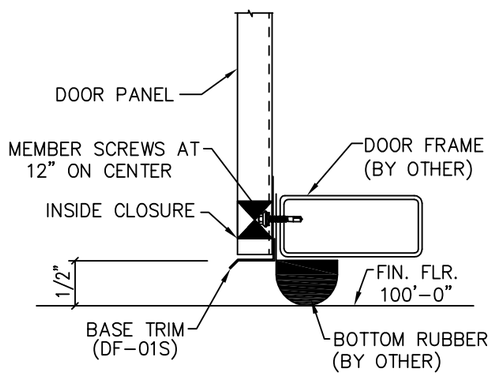
PANELS: 26 Ga. PBR – ASH GRAY
NOTE: FIELD CUT DOOR WALL PANELS AS REQUIRED



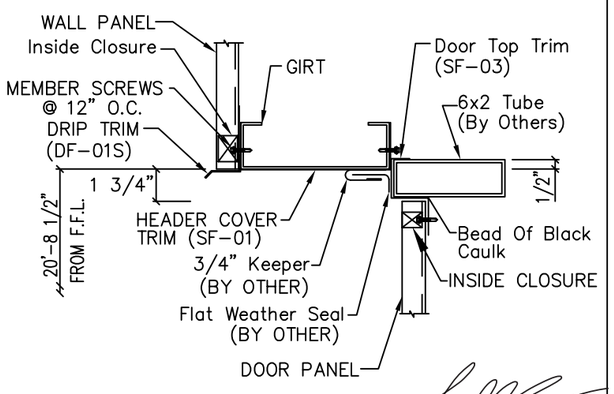
TRIM 1X HEADER TRIM AT WIDE OPENING



TRIM 2X JAMB TRIM DETAIL AT HYDRAULIC DOOR



TRIM 3X BASE TRIM AT HYDRAULIC DOOR
PBR PANEL



TRIM 4X HEADER TRIM DETAIL AT HYDRAULIC DOOR
PBR PANEL

06/11/2024

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

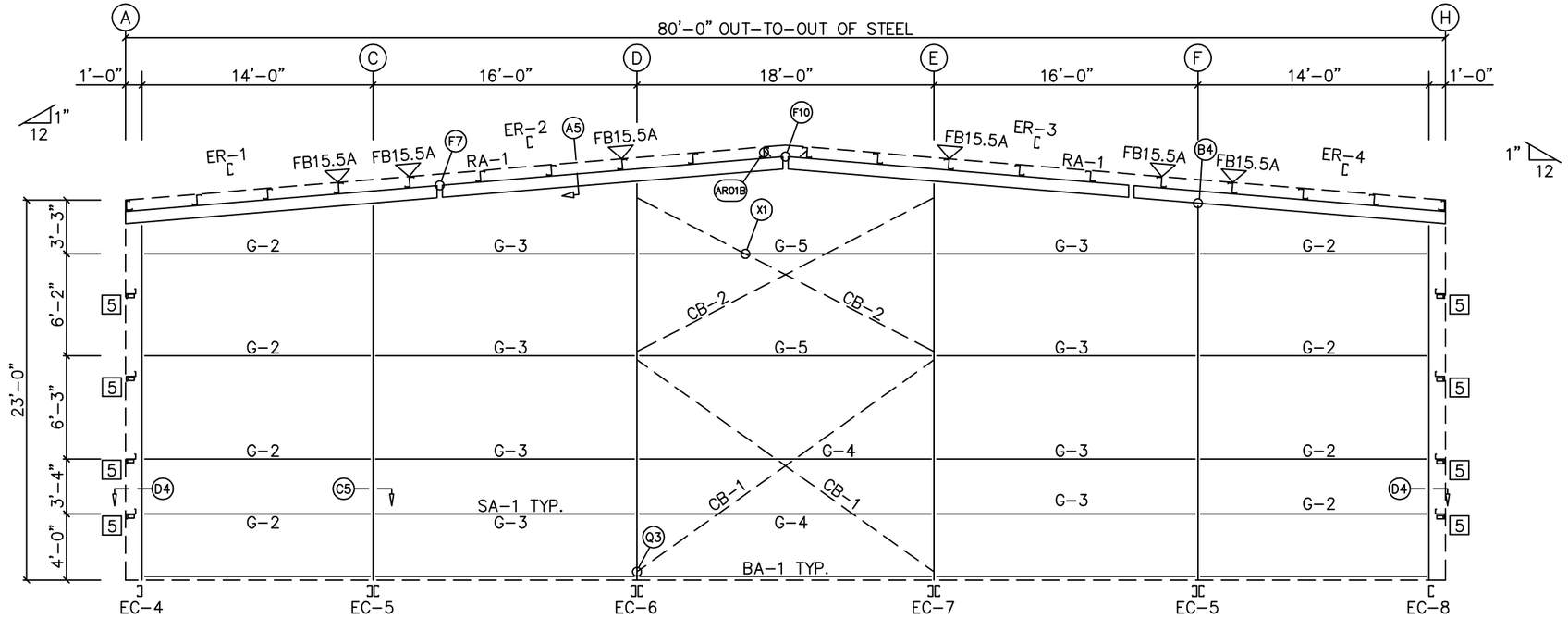
METAL BUILDING OUTLET CORP.

DESCRIPTION	HP HYDRAULIC DOOR SHEETING
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	E6 OF E9

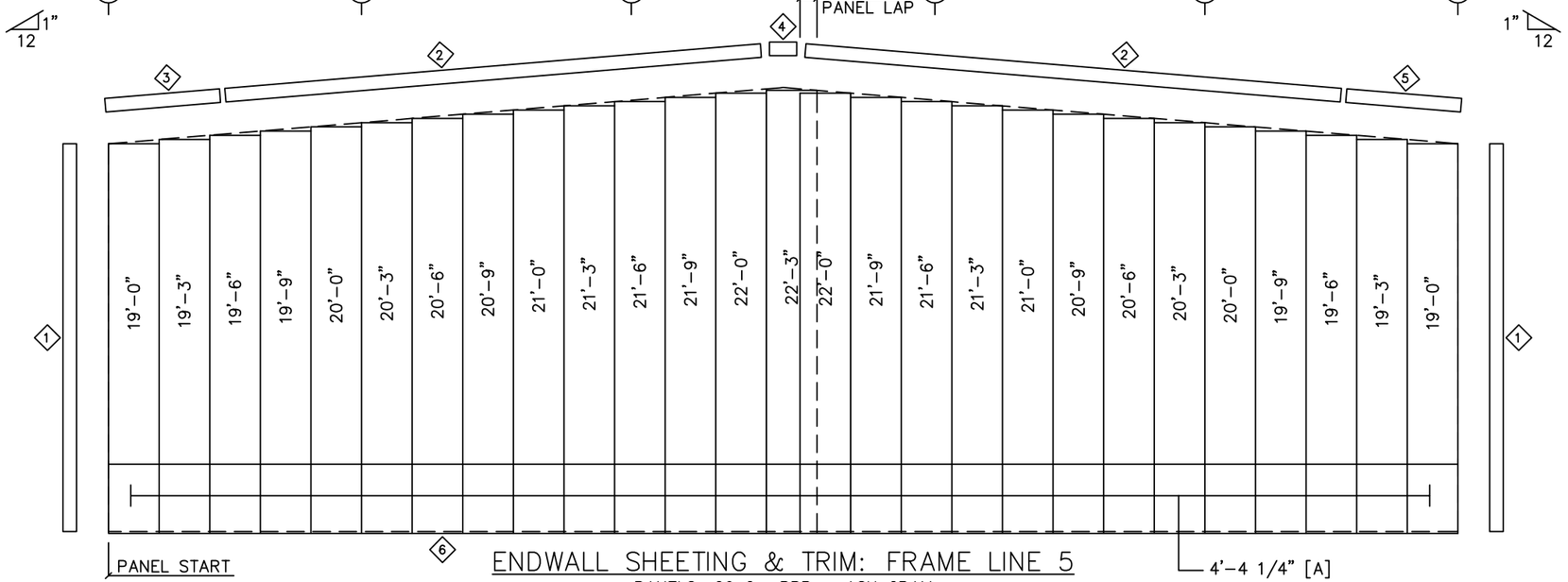
FOR CONSTRUCTION

REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH



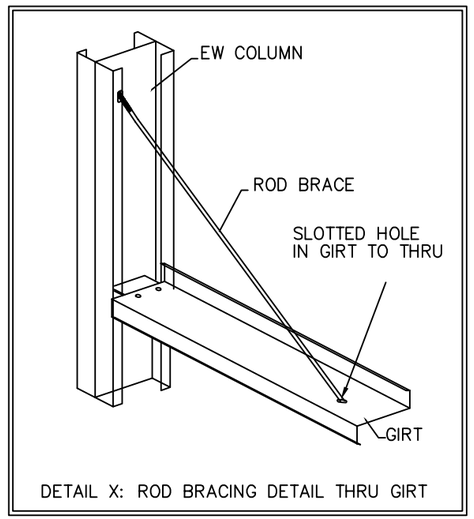
ENDWALL FRAMING: FRAME LINE 5

4" WALL INSULATION (BY OTHERS)



ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 Ga. PBR – ASH GRAY
[A] PANELS: 26 Ga. PBR – CHARCOAL GRAY
(FIELD CUT WALL PANELS AS REQUIRED)



DETAIL X: ROD BRACING DETAIL THRU GIRT

TRIM TABLE
FRAME LINE 5

◇ID	QUAN	PART	LENGTH	COLOR	DETAIL
1	4	CT-01R	12'-0"	ASH GRAY	TRIM_19
2	4	RT-01	15'-7"	CHARCOAL GRAY	TRIM_109
3	1	RT-01L	11'-1"	CHARCOAL GRAY	TRIM_35
4	1	PB-50	1'-6"	CHARCOAL GRAY	TRIM_12
5	1	RT-01R	11'-1"	CHARCOAL GRAY	TRIM_109
6	8	DF-01R	10'-6"	CHARCOAL GRAY	TRIM_176

BOLT TABLE
FRAME LINE 5

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	4	A325	5/8"	1 3/4"
ER-2/ER-3	4	A325	5/8"	1 3/4"
ER-3/ER-4	4	A325	5/8"	1 3/4"
Columns/Raf	4	A325	1/2"	1 1/4"

FLANGE BRACE TABLE
FRAME LINE 5

▽ID	MARK	LENGTH
1	FB15.5A	1'-3 1/2"

CONNECTION PLATES
FRAME LINE 4

□ID	QUAN	MARK/PART
5	8	CC5

MEMBER TABLE
FRAME LINE 5

MARK	PART
EC-4	8X25C12
EC-5	8X50D12
EC-6	8X50D12
EC-7	8X50D12
EC-8	8X25C12
ER-1	10X25C12
ER-2	10X25C12
ER-3	10X25C12
ER-4	10X25C12
G-2	8x25Z16
G-3	8x25Z16
G-4	8x25Z16
G-5	8x25Z12
CB-1	BR1/2
CB-2	BR1/2

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/06/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

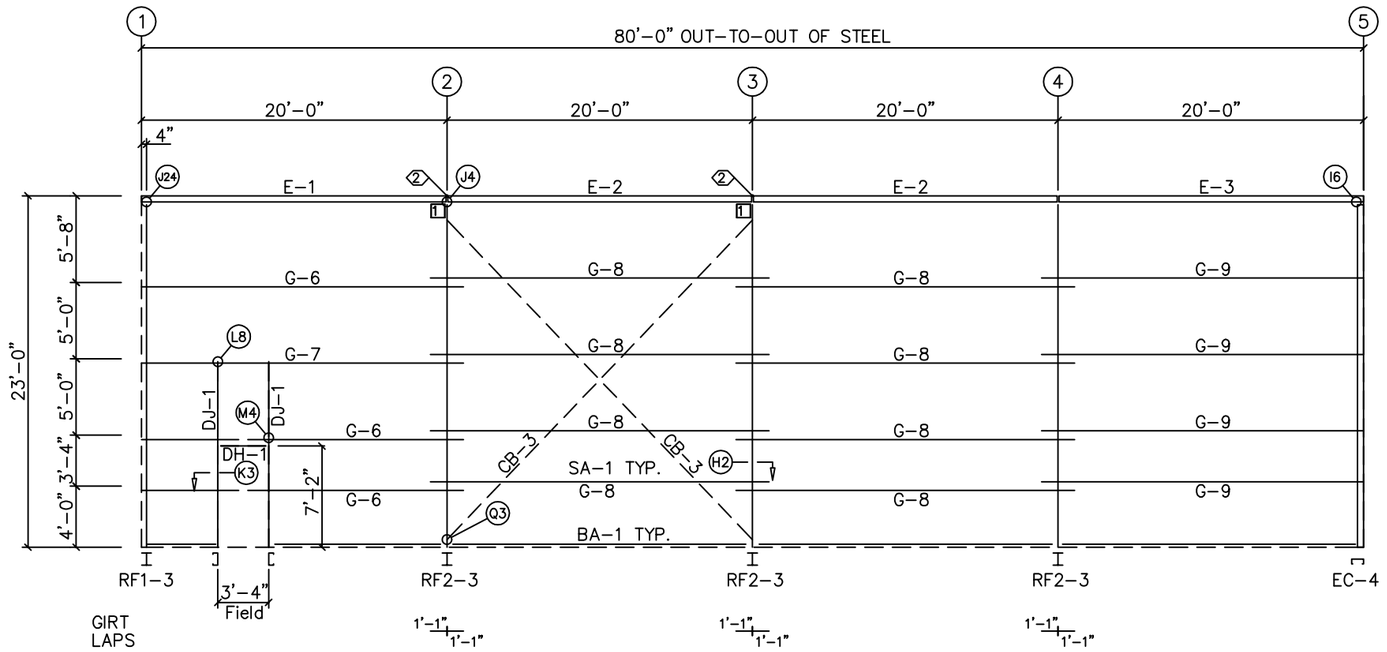
DESCRIPTION	ENDWALL FRAMING & SHEETING
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.# 112949 JOB# 112949 SCALE N.T.S. DWG# E7 OF E9

FOR CONSTRUCTION

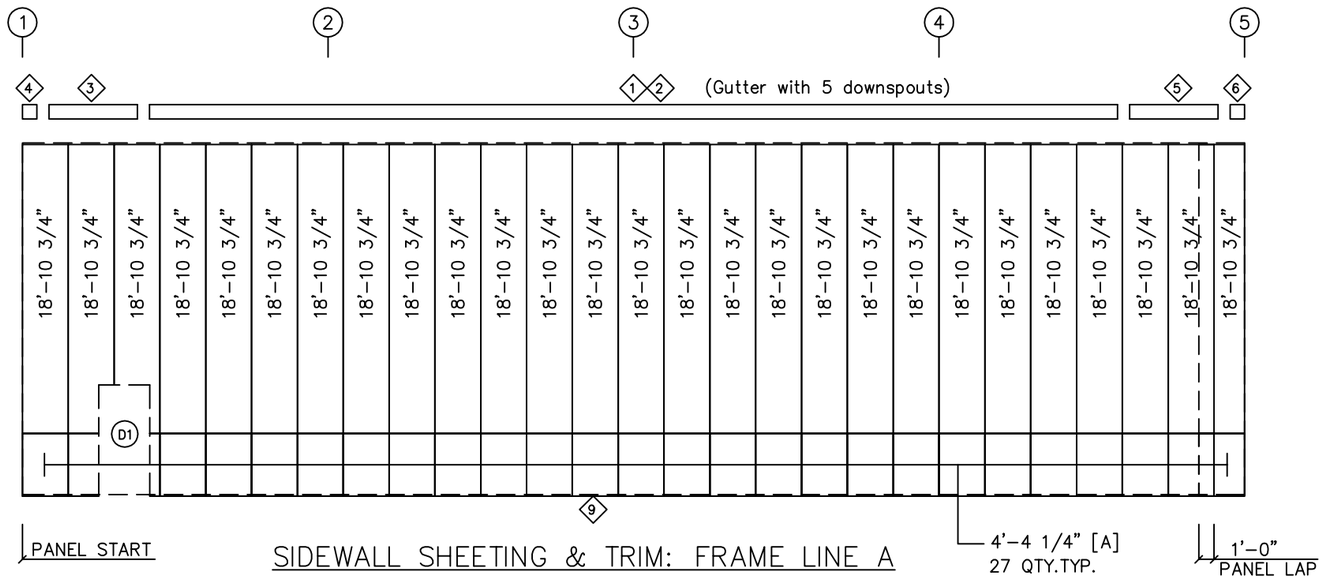
06/11/2024

REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH



SIDEWALL FRAMING: FRAME LINE A

4" WALL INSULATION (BY OTHERS)

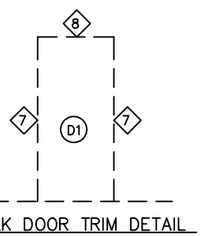


SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. PBR – ASH GRAY

[A] PANELS: 26 Ga. PBR – CHARCOAL GRAY

(FIELD CUT WALL PANEL AS REQUIRED)



WALK DOOR TRIM DETAIL
3'-4"x7'-2"

TRIM TABLE
FRAME LINE A

◇ID	QUAN	PART	LENGTH	COLOR	DETAIL
1	4	PG-01A	15'-6"	CHARCOAL GRAY	TRIM_46
2	8	PCT-01	10'-6"	CHARCOAL GRAY	TRIM_46
3	1	PG-01AL	11'-1"	CHARCOAL GRAY	TRIM_35
4	1	EC-01AL	9"	CHARCOAL GRAY	TRIM_17
5	1	PG-01AR	11'-1"	CHARCOAL GRAY	TRIM_35
6	1	EC-01AR	9"	CHARCOAL GRAY	TRIM_17
7	2	JT-01R	7'-4"	CHARCOAL GRAY	TRIM_80
8	1	HT-01R	3'-8"	CHARCOAL GRAY	TRIM_72
9	8	DF-01R	10'-6"	CHARCOAL GRAY	TRIM_176

SPECIAL BOLTS

◇ ID	QUAN	TYPE	DIA	LENGTH	WASH
2	4	A325	1/2"	1 1/4"	0

CONNECTION PLATES
FRAME LINE A

□ID	QUAN	MARK/PART
1	2	CC12A

MEMBER TABLE
FRAME LINE A

MARK	PART
DJ-1	8X25C14
DH-1	8x25c16
E-1	E0852.7541L
E-2	E0852.7541L
E-3	E0852.7541L
G-6	8x25Z16
G-7	8x25Z14
G-8	8x25Z16
G-9	8x25Z16
CB-3	BR3/4

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

DESCRIPTION	SIDEWALL FRAMING & SHEETING
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	E8 OF E9

FOR CONSTRUCTION

06/11/2024

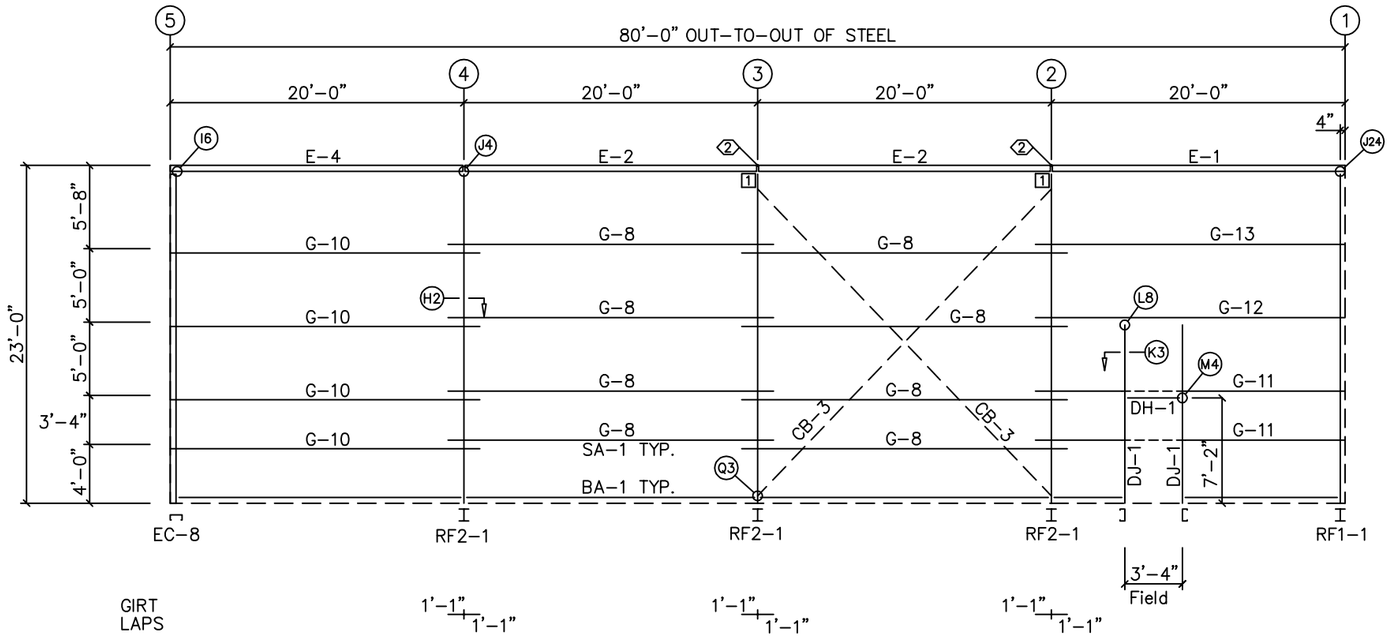
REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH

TRIM TABLE FRAME LINE H					
◇ID	QUAN	PART	LENGTH	COLOR	DETAIL
1	4	PG-01A	15'-6"	CHARCOAL GRAY	TRIM_46
2	8	PCT-01	10'-6"	CHARCOAL GRAY	TRIM_46
3	1	PG-01AL	11'-1"	CHARCOAL GRAY	TRIM_35
4	1	EC-01AL	9"	CHARCOAL GRAY	TRIM_17
5	1	PG-01AR	11'-1"	CHARCOAL GRAY	TRIM_35
6	1	EC-01AR	9"	CHARCOAL GRAY	TRIM_17
7	2	JT-01R	7'-4"	CHARCOAL GRAY	TRIM_80
8	1	HT-01R	3'-8"	CHARCOAL GRAY	TRIM_72
9	8	DF-01R	10'-6"	CHARCOAL GRAY	TRIM_176

SPECIAL BOLTS					
◇ ID	QUAN	TYPE	DIA	LENGTH	WASH
2	4	A325	1/2"	1 1/4"	0

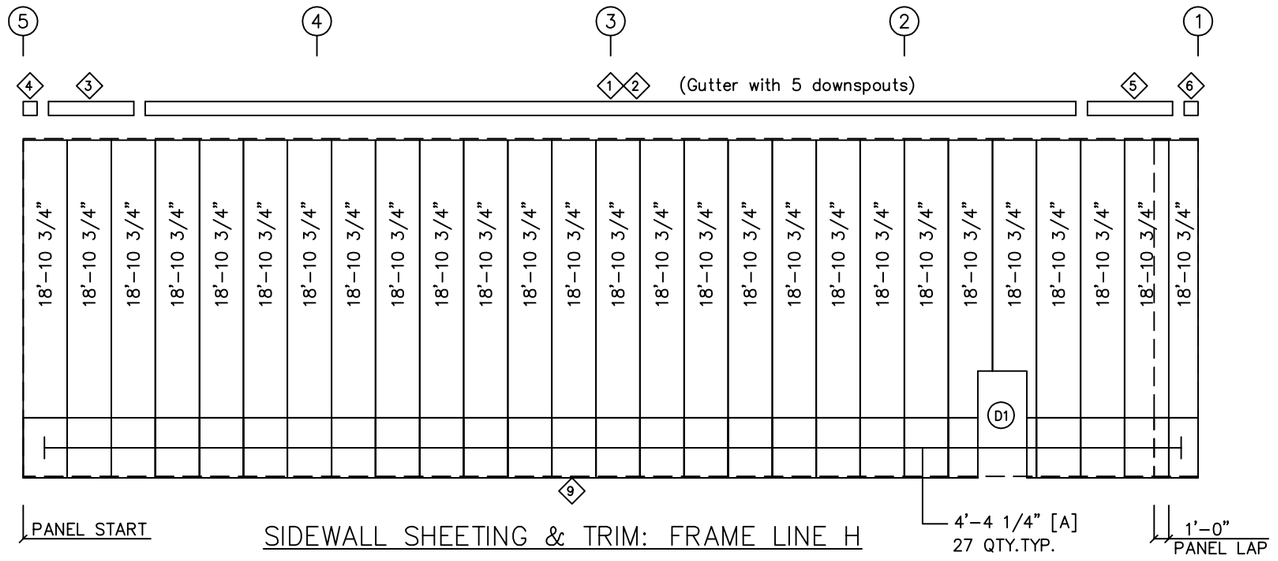
CONNECTION PLATES FRAME LINE H		
□ID	QUAN	MARK/PART
1	2	CC12A

MEMBER TABLE FRAME LINE H	
MARK	PART
DJ-1	8X25C14
DH-1	8x25c16
E-1	E0852.7541L
E-2	E0852.7541L
E-4	E0852.7541L
G-8	8x25Z16
G-10	8x25Z16
G-11	8x25Z16
G-12	8x25Z16
G-13	8x25Z16
CB-3	BR3/4



SIDEWALL FRAMING: FRAME LINE H

4" WALL INSULATION (BY OTHERS)

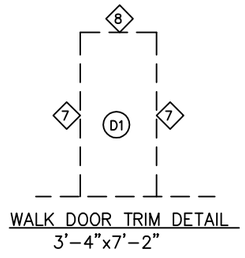


SIDEWALL SHEETING & TRIM: FRAME LINE H

PANELS: 26 Ga. PBR – ASH GRAY

[A] PANELS: 26 Ga. PBR – CHARCOAL GRAY

(FIELD CUT WALL PANEL AS REQUIRED)



WALK DOOR TRIM DETAIL
3'-4"x7'-2"

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

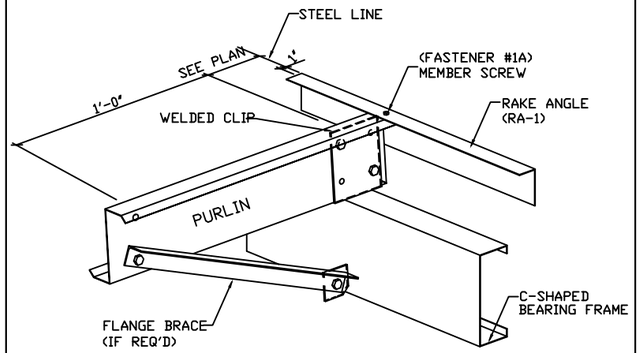
DESCRIPTION	SIDEWALL FRAMING & SHEETING
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	E9 OF E9

FOR CONSTRUCTION

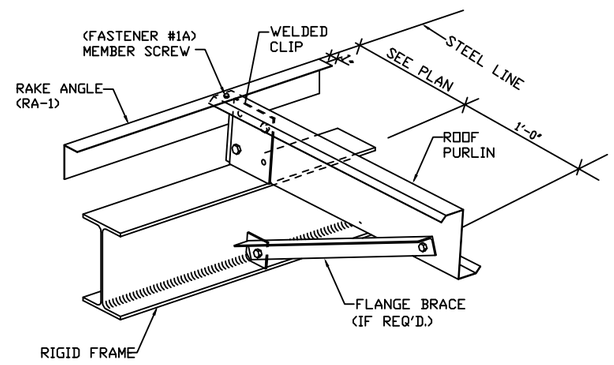
06/11/2024

REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH



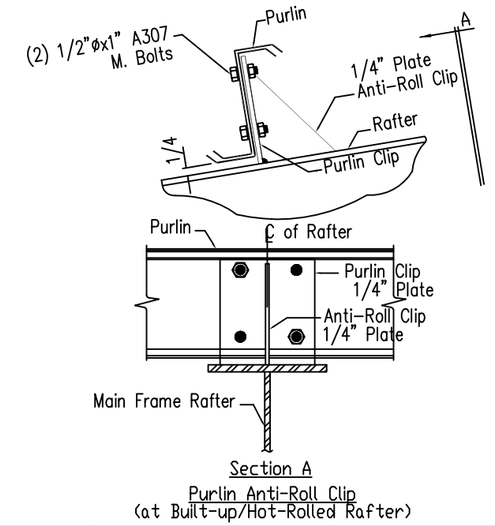
A5 BEARING FRAME ENDWALL SECTION

ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS TYP.



A10 ROOF PURLIN TO EXPANDABLE ENDWALL RIGID FRAME

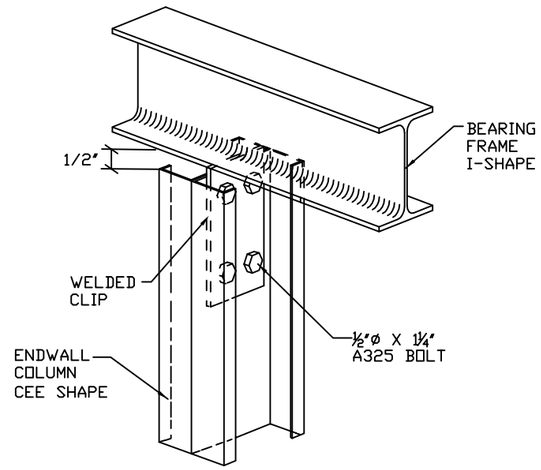
ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS TYP.



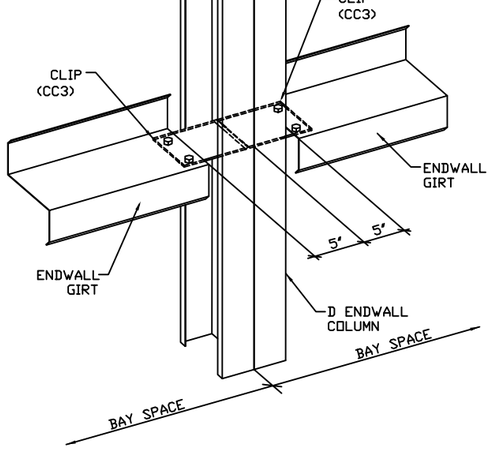
Section A

Purlin Anti-Roll Clip
(at Built-up/Hot-Rolled Rafter)

AR01B ANTI-ROLL CLIP AT MAIN FRAME

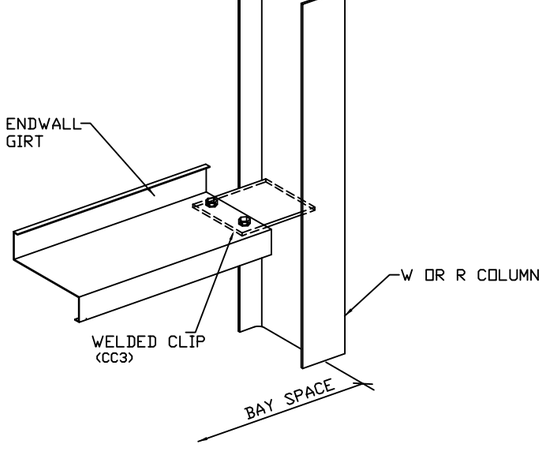


B4 ENDWALL RAFTER TO CEE COLUMN



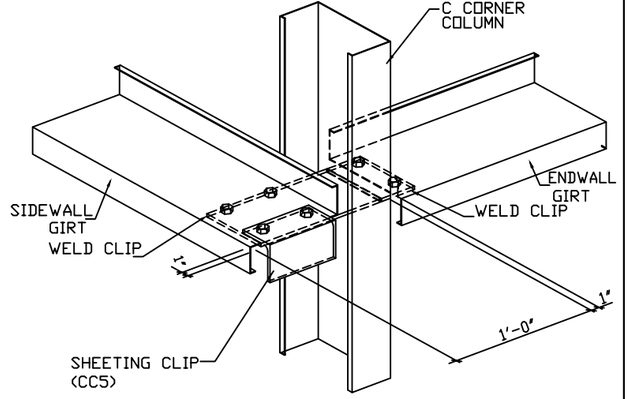
C5 DOUBLE CEE ENDWALL COLUMN

ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS TYP.



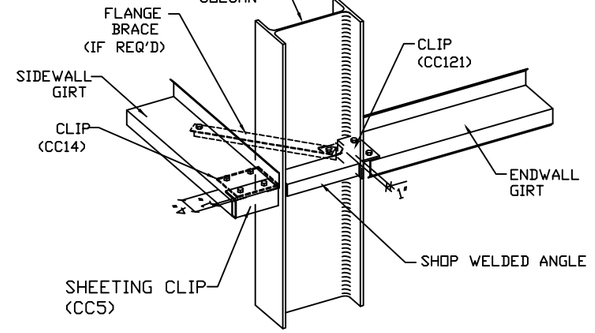
C15 GIRT/ HEADER BEAM TO W OR R COLUMN

ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS TYP.



D4 SINGLE CEE CORNER COLUMN

ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS TYP.



D15 RIGID FRAME CORNER COLUMN TO WALL GIRT

ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS TYP.

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

DESCRIPTION	DETAIL DRAWINGS
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	S1 OF S7

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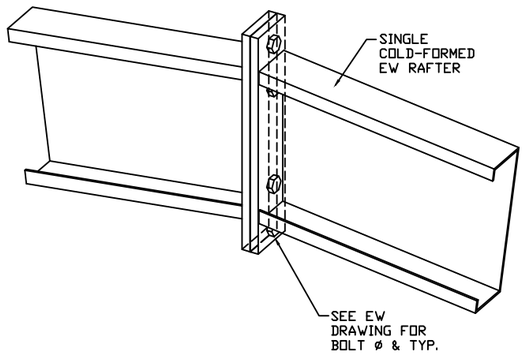
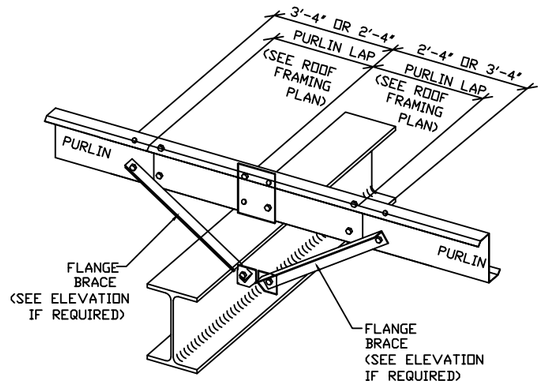
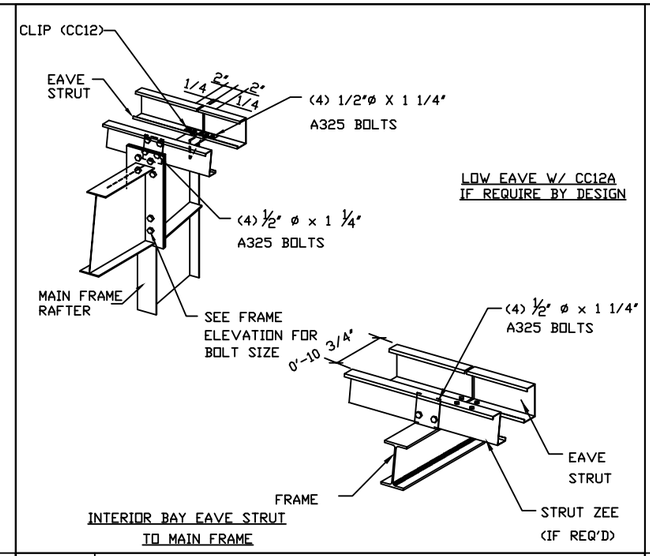


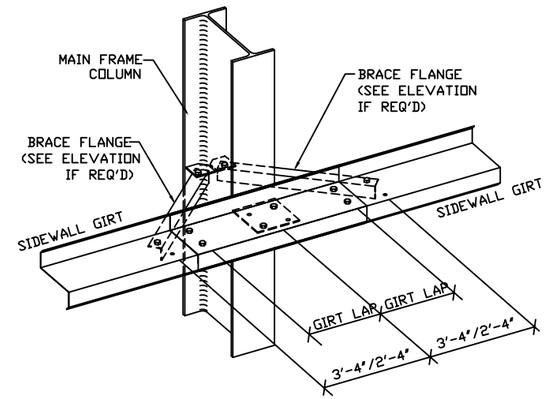
F10	RAFTER SPLICE AT RIDGE SINGLE COLD - FORMED RAFTER
	SEE ENDWALL DRAFTING FOR BOLT AND TYPE.



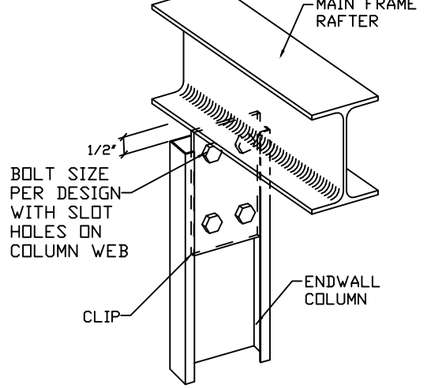
G2	BY-PASS PURLIN TO RAFTER DETAIL
	ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS



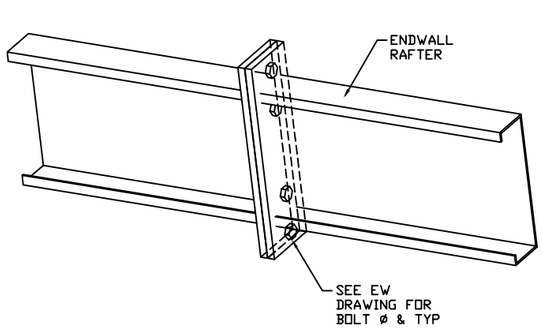
J4	LOW EAVE DETAIL (BYPASS CONDITION)
	ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS



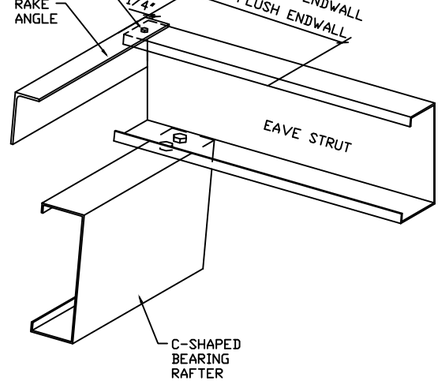
H2	MAIN FRAME INTERIOR COLUMN BY-PASS GIRTS
	ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS TYP.



B19	ENDWALL COLUMN TO MF RAFTER



F7	RAFTER SPLICE ALONG SURFACE
	SEE ENDWALL DRAFTING FOR BOLT AND TYPE



I6	END BAY EAVE STRUT TO BEARING FRAME ENDWALL
	ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS TYP

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METAL BUILDING OUTLET CORP.

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END USER	RUSSEL KEY 52F
END USE	Aviation
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CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	S2 OF S7

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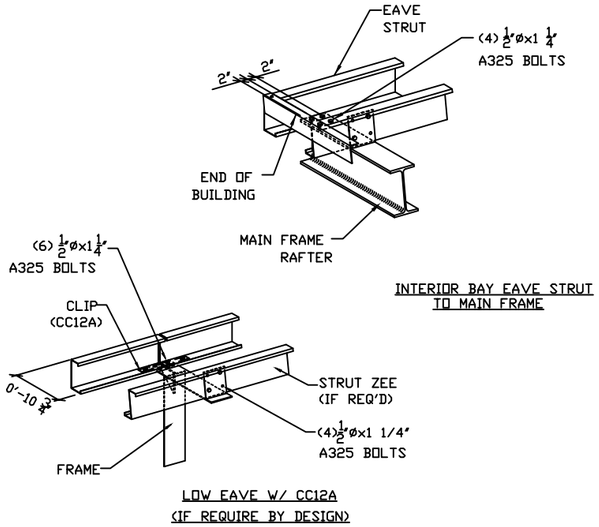
STATE OF UTAH

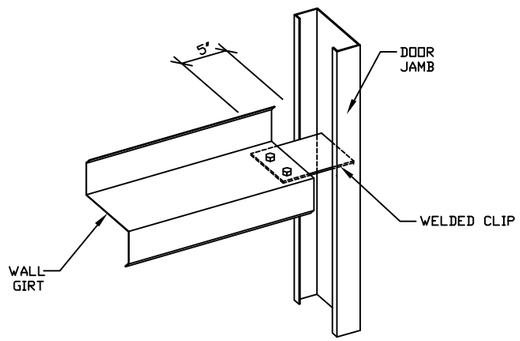


INTERIOR BAY EAVE STRUT TO MAIN FRAME

LOW EAVE W/ CC12A (IF REQUIRE BY DESIGN)

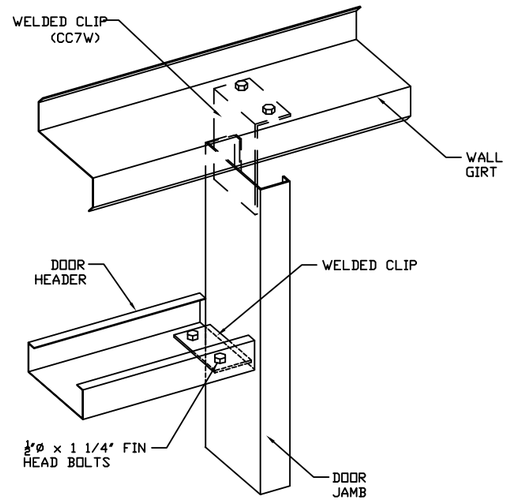
J24 LOW EAVE STRUT TO RIGID FRAME (BYPASS CONDITION)

ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS



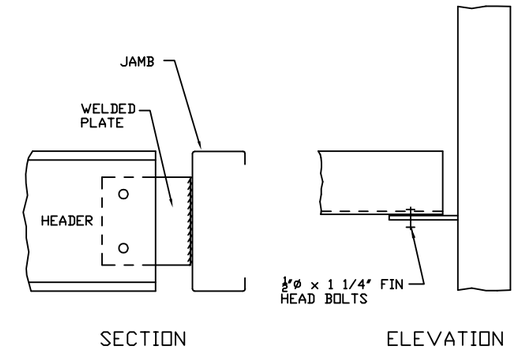
K3 WALL GIRT TO DOOR JAMB

ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS



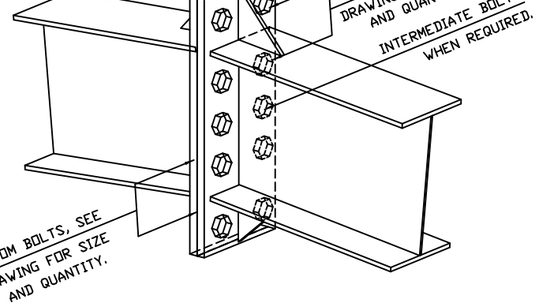
L8 DOOR JAMB TO WALL GIRT WELDED CLIPS

ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS



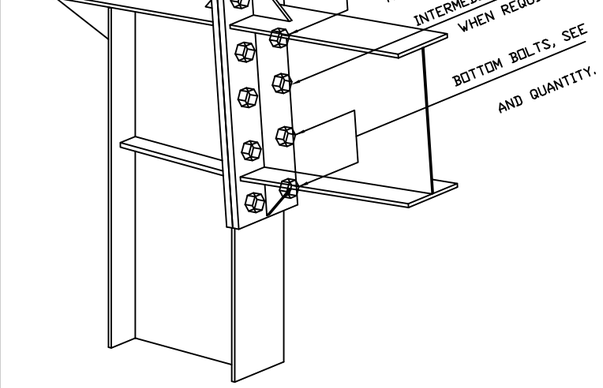
M4 HEADER TO C JAMB

ALL BOLTS ARE 1/2"ø x 1 1/4" A325 BOLTS



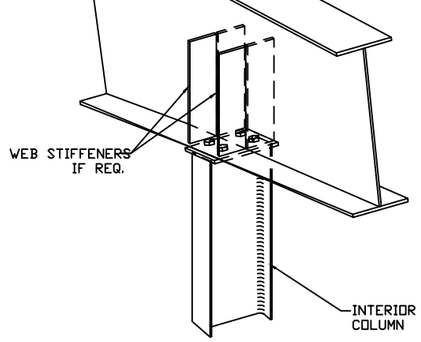
U2 BOLTED PLATE CONNECTION AT PEAK

SEE SPLICE PLATE & BOLT TABLE



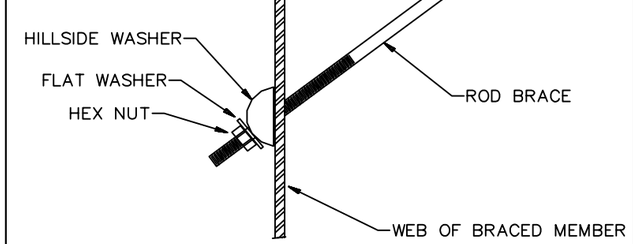
U3 BOLTS FOR RAFTER TO COLUMN CONNECTION

SEE SPLICE PLATE & BOLT TABLE



*SEE ERECTION DRAWINGS FOR BOLT SIZE.

V2 INTERIOR COLUMN TO RAFTER



Q3 ROD BRACE TO WEB CONNECTION

NOTE! USE A BACKUP PLATE UNDER THE HILLSIDE WASHER FOR COLD FORM MEMBERS

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A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
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METAL BUILDING OUTLET CORP.

DESCRIPTION	DETAIL DRAWINGS
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END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	S3 OF S7

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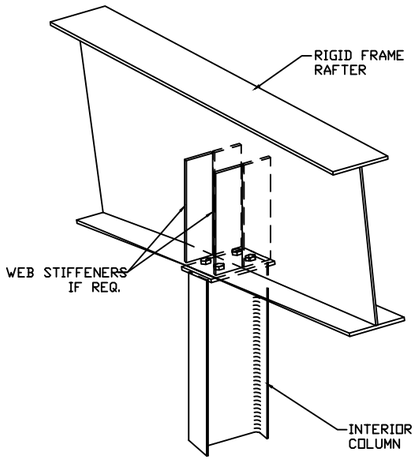
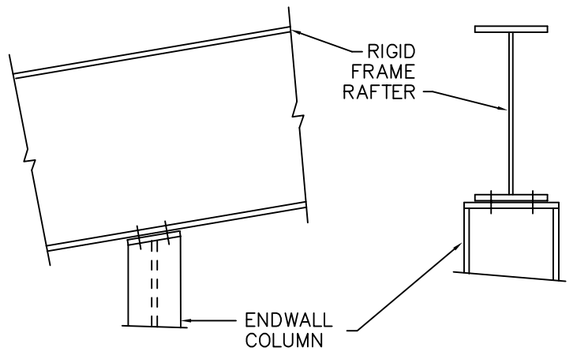


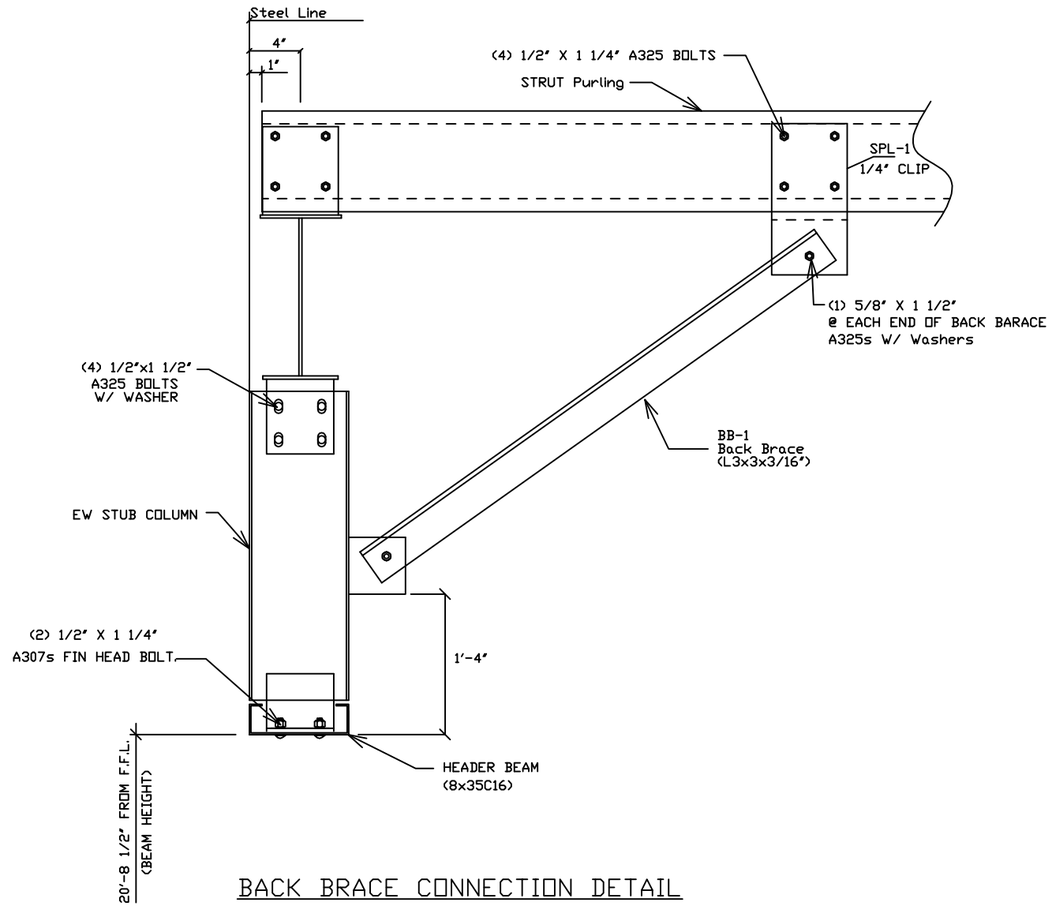
*SEE ERECTION DRAWINGS FOR BOLT SIZE.

V2 INTERIOR COLUMN TO RAFTER



* SEE ERECTION DRAWINGS FOR BOLT SIZE.

B20 HOTROLLED COLUMN TO RIGID FRAME RAFTER



BACK BRACE CONNECTION DETAIL

SECTION-"A/S4"

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

DESCRIPTION	DETAIL DRAWINGS
BUYER / CUSTOMER	RUSSEL KEY 52F
END USER	RUSSEL KEY 52F
END USE	Aviation
STREET	4550 S AIRPORT LOT 52F
CITY, STATE, ZIP	ST GEORGE UT 84790
COUNTY	WASHINGTON

S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	S4 OF S7

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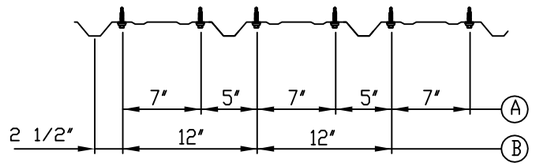
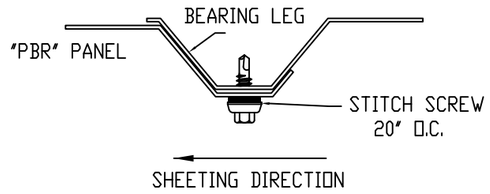


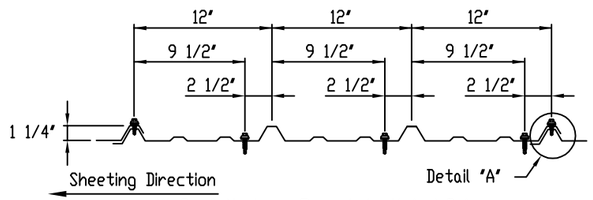
PBR PANEL

(A) = AT BASE, RAKE, EAVE, MID SPAN END LAPS

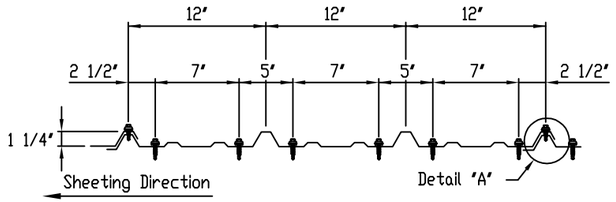
(B) = AT INTERMEDIATE MEMBER, OPTIONAL LINER PANEL



SCREW	
1	PBR WALL FASTENER LOCATION



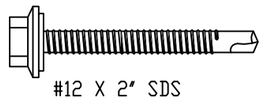
All Roof Members Except As Noted Below



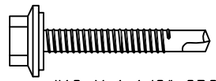
At Eave Strut, Panel End Lap, and Peak Purlin

SCREW	
2	PBR ROOF FASTENER LOCATION

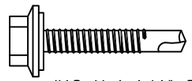
MEMBER SCREWS
(SHORT, MEDIUM, OR LONG)



#12 X 2" SDS
W/ WASHER

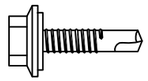


#12 X 1 1/2" SDS
W/ WASHER



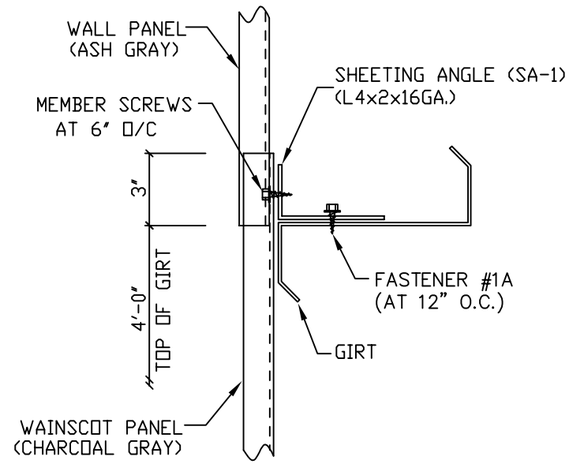
#12 X 1 1/4" SDS
W/ WASHER

STITCH

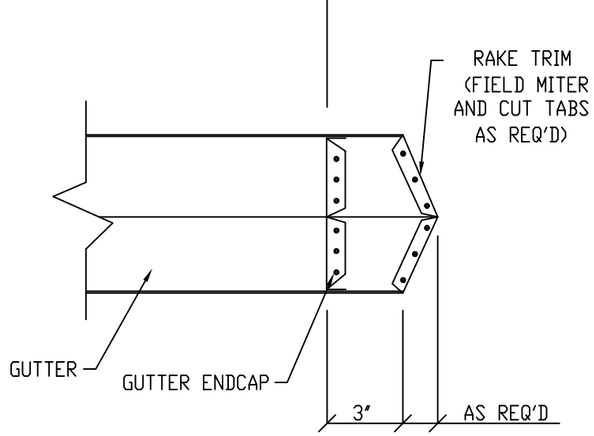


#14 X 7/8" LAP-TEK
W/ WASHER

SCREW	
4	FASTENERS SELF DRILLING

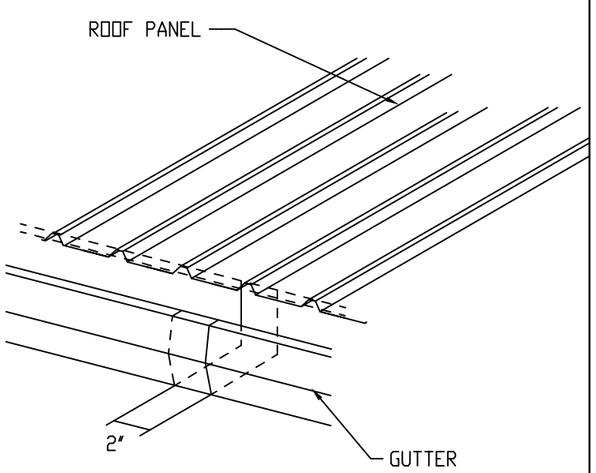


TRIM	
18	WAINSCOT PANEL LAP DETAIL ("PBR" WALL PANEL)

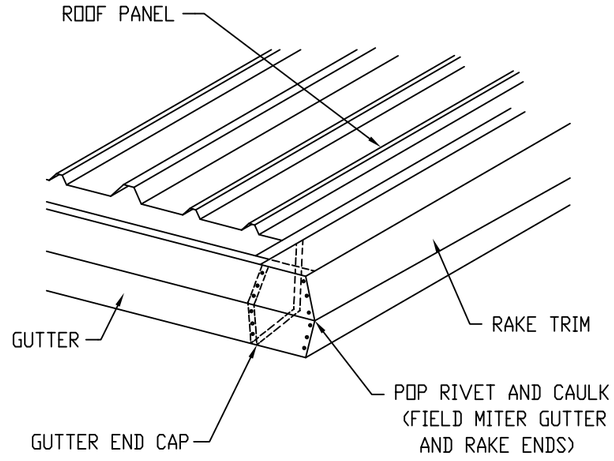


TRIM	
17	EAVE TO RAKE TRANSITION TYPICAL GUTTER

GUTTER SHOWN, RAKE TRIM SIM.



TRIM	
15	TRIM LAPS



TRIM	
35	EAVE TO RAKE TRANSITION TYPICAL GUTTER

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
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METAL BUILDING OUTLET CORP.

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END USER	RUSSEL KEY 52F						
END USE	Aviation						
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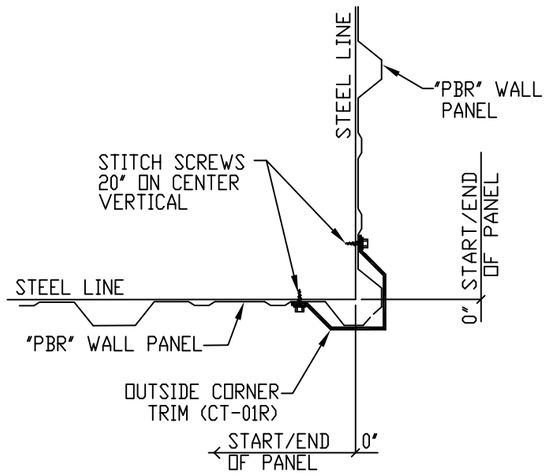
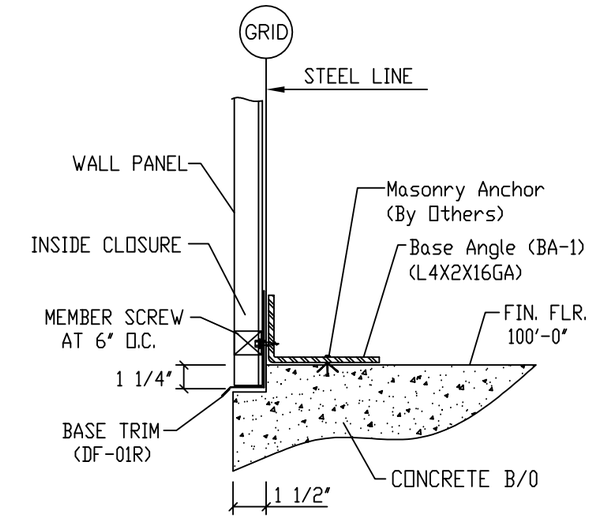
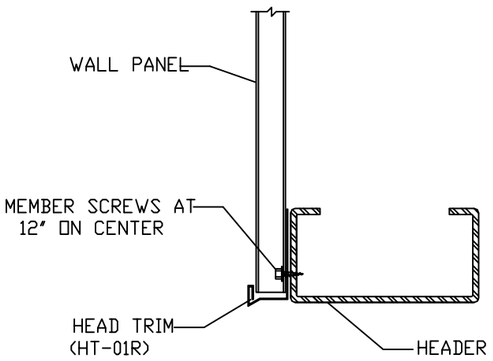
FREDERICK J. CAMPANA
STATE OF UTAH



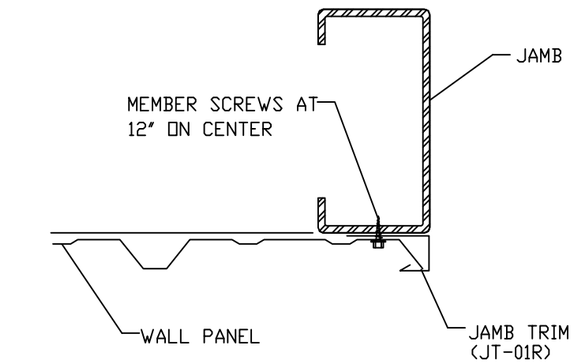
TRIM	OUTSIDE CORNER DETAIL PBR WALL PANEL
19	



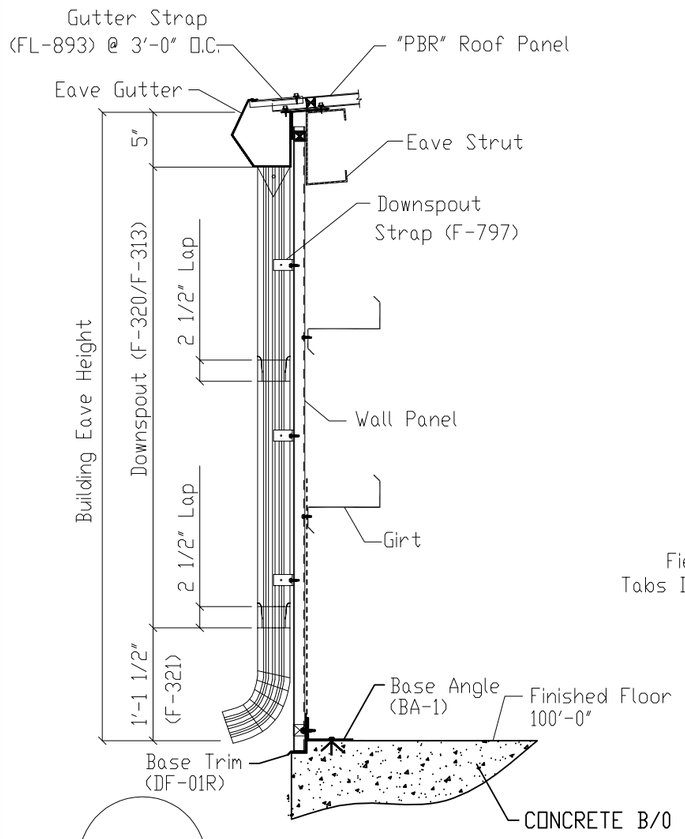
TRIM	BASE DETAIL WITH CONCRETE NOTCH SHEETED WALL WITH BASE ANGLE
176	



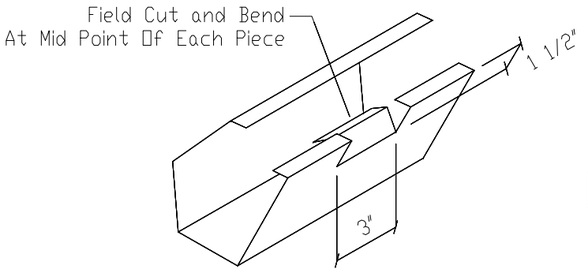
TRIM	HEADER DETAIL FOR FRAMED OPENINGS ("PBR" WALL PANEL)
72	



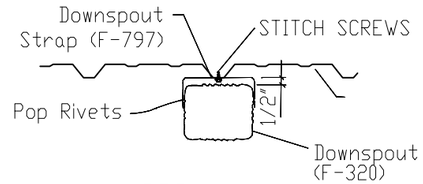
TRIM	JAMB DETAIL FOR FRAMED OPENINGS ("PBR" WALL PANEL)
80	



TRIM_60 ROLLED DOWNSPOUT AT SHEETED WALL

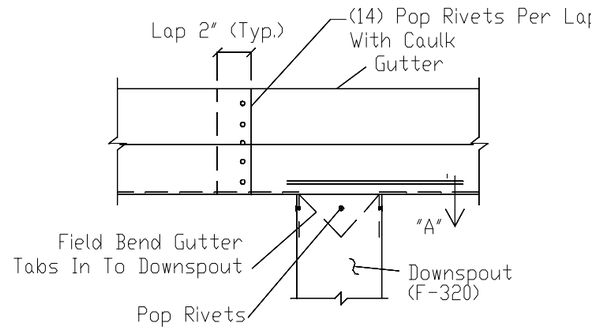


GUTTER OVERFLOW RELIFE

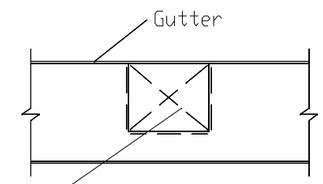


"PBR" WALL

DOWNSPOUT STRAP ATTACHMENT



DOWNSPOUT TO GUTTER DETAIL



SECTION "A"

D.S. STRAP QUANTITY @ 6'-0" O.C. MAX

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

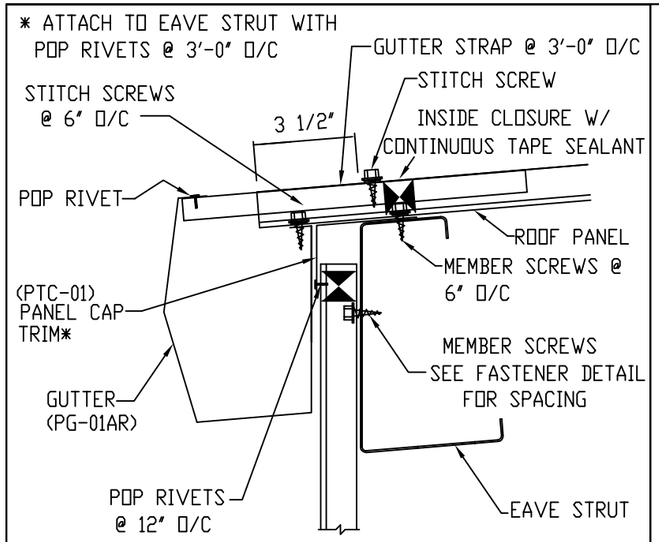
METAL BUILDING OUTLET CORP.

DESCRIPTION	DETAIL DRAWINGS						
BUYER / CUSTOMER	RUSSEL KEY 52F						
END USER	RUSSEL KEY 52F						
END USE	Aviation						
STREET	4550 S AIRPORT LOT 52F						
CITY, STATE, ZIP	ST GEORGE UT 84790						
COUNTY	WASHINGTON						
S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	S6 OF S7

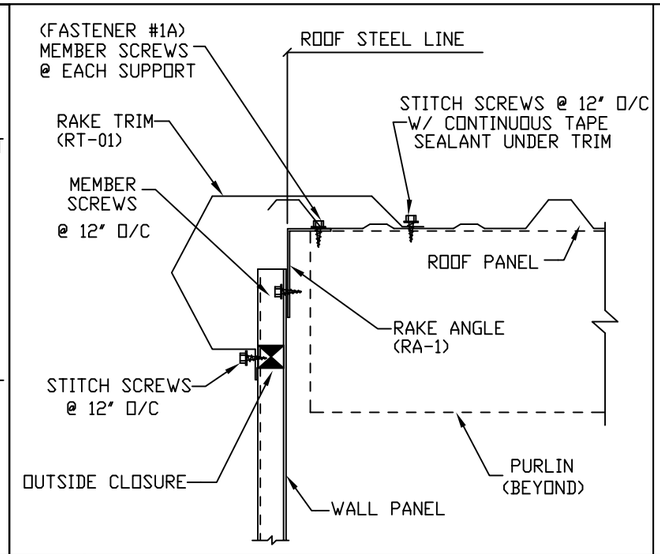
FOR CONSTRUCTION

06/11/2024

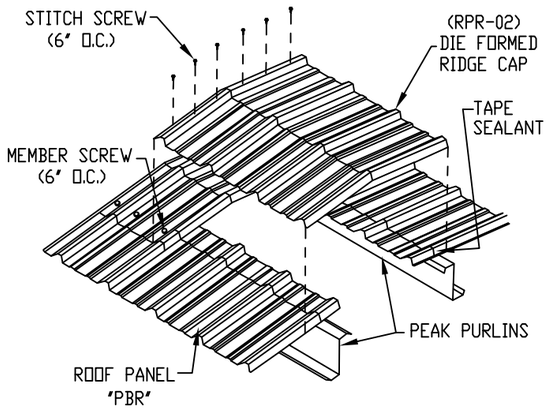
REGISTERED PROFESSIONAL ENGINEER
No. 378230
FREDERICK J. CAMPANA
STATE OF UTAH

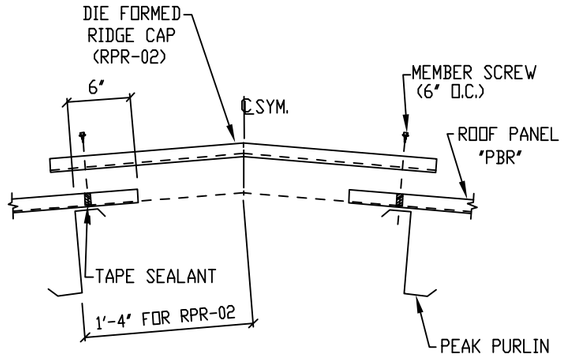


TRIM	LOW EAVE DETAIL STANDARD GUTTER SHEETED WALL
46	

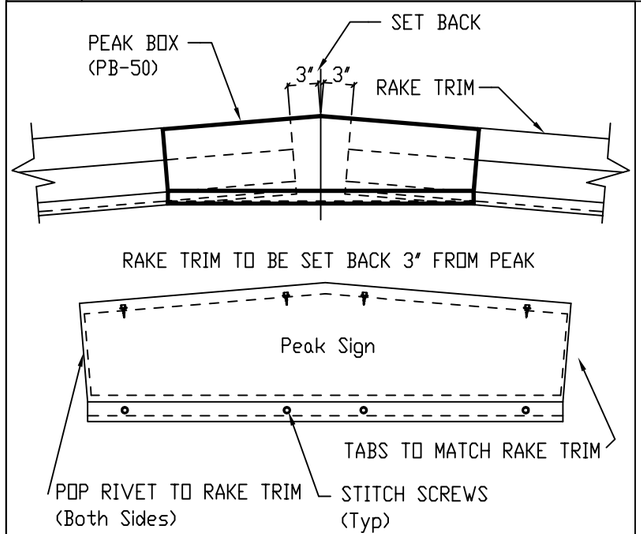


TRIM	RAKE TRIM DETAIL SHEETED WALL
109	





TRIM	DIE FORMED RIDGE DETAIL- 'PBR"
13	



TRIM	PEAK BOX INSTALLATION
12	

ISSUE	DESCRIPTION	DATE	DRN	CHK	DES
A	FOR PERMIT	05/08/2024	VBA	RAK	GFA
0	FOR CONSTRUCTION	05/10/2024	VBA	RAK	GFA

METAL BUILDING OUTLET CORP.

DESCRIPTION	DETAIL DRAWINGS						
BUYER / CUSTOMER	RUSSEL KEY 52F						
END USER	RUSSEL KEY 52F						
END USE	Aviation						
STREET	4550 S AIRPORT LOT 52F						
CITY, STATE, ZIP	ST GEORGE UT 84790						
COUNTY	WASHINGTON						
S.O.#	112949	JOB#	112949	SCALE	N.T.S.	DWG#	S7 OF S7

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FOR CONSTRUCTION

06/11/2024

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